

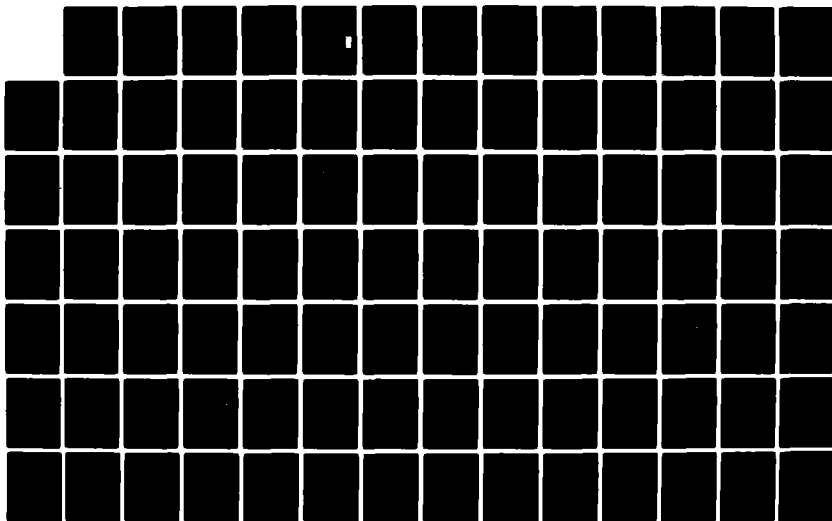
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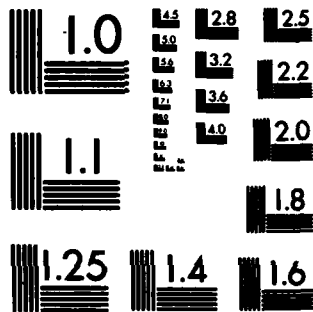
ANALYSIS OF PROGRESSIVE COLLAPSE OF COMPLEX STRUCTURES
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Name: Gregory Edward Riggs

Date of Degree: December, 1982

Institution: Oklahoma State University

Location: Stillwater, Oklahoma

Title of Study: ANALYSIS OF PROGRESSIVE COLLAPSE OF COMPLEX STRUCTURES

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Candidate for Degree of Doctor of Philosophy

Major Field: Civil Engineering

Scope and Method of Study: The principal goal of the study was to evaluate an analytical procedure for predicting progressive collapse in damaged complex structures. A structure was modeled for analysis by the finite element method using relatively large, simple elements. There was little or no refinement of mesh size in areas of initial damage or damage propagation. A method was developed for determining and applying allowable stresses to help compensate for the absence of model detail. Stress results of a finite element analysis were examined by a computer post-processor program written for this study to make selective changes to the finite element model. The modified model was analyzed using the finite element method and the procedure was repeated in an iterative fashion to predict progressive collapse. Analytical results were compared to experimental test data to determine the validity of the analytical procedure.

Findings and Conclusions: The analytical procedure provided a relatively economical method for predicting progressive collapse in a complex structure. Evaluation of a complex structure subjected to three initial damage conditions showed acceptable correlation between experimental and analytical results. The method of determining appropriate allowable stresses was general enough to apply to a wide range of materials and structures. The procedure proved to be an economical estimating tool for predicting residual structural strength.



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ANALYSIS OF PROGRESSIVE COLLAPSE
OF COMPLEX STRUCTURES

By

GREGORY EDWARD RIGGS

Bachelor of Science in Civil Engineering
United States Air Force Academy
Colorado Springs, Colorado
1972

Master of Science
University of Illinois
Urbana, Illinois
1973

Submitted to the Faculty of the Graduate College
of the Oklahoma State University
in partial fulfillment of the requirements
for the Degree of
DOCTOR OF PHILOSOPHY
December, 1982

ANALYSIS OF PROGRESSIVE COLLAPSE
OF COMPLEX STRUCTURES

Thesis Approved:

Thesis Adviser

Dean of the Graduate College

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I thank the Joint Technical Coordinating Group for Munitions Effectiveness for funding this study. I sincerely hope this work will provide long-term benefits for the Group in performing its mission.

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Ms. Charlene Fries receives thanks from students every semester for typing manuscripts, but such thanks severely underrate her contributions. She assumes full responsibility for administering the preparation and submission of the documents. I certainly thank her.

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CHAPTER I

INTRODUCTION

Many structures are susceptible to progressive collapse, a chain reaction type of failure following damage to a relatively small portion of the structure. The more specialized a structure is, the more vulnerable it is to progressive failure largely because it is designed to resist fewer possible loading conditions. As efforts increase to optimize designs within acceptable factors of safety, the risks of initiating progressive collapse through relatively minor localized damage also increase. An ability to predict analytically the response of a damaged structure would therefore be beneficial.

Although progressive collapse is normally associated with high-rise buildings, interest in it is not limited to conventional civil engineering applications. The Department of Defense needs the capability to predict the residual strength of battle-damaged aircraft and to know the role of progressive collapse in that setting. Specifically, the Department of Defense Joint Technical Coordinating Group for Munitions Effectiveness is interested in the post-damaged capabilities of potentially hostile aircraft.

In pursuit of its interest, the Group provided research funds and three F-84F aircraft wings for this study. The goal was to evaluate a potentially versatile method for predicting progressive collapse in aircraft structures. The method was to be verified by experimental testing.

Desirable characteristics to be imparted to the method would be relative simplicity in preparing for its use and relative ease and economy in its application.

The finite element method was the fundamental tool for determining stresses within the wing. In this report most discussion of the finite element method is of a general nature. The NASTRAN (National Aeronautical and Space Administration Structural Aalysis) program was selected to apply the finite element method because of its versatility and its widespread availability in both industry and the defense community. The reader is assumed to be familiar with the finite element method in general. Where reference to specific program characteristics is essential, a basic familiarity with NASTRAN is also assumed.

Finally, the sponsor of this research is interested in the effectiveness of munitions in destroying combat aircraft. Consequently, any conservative assumption is one which tends to give the structure more strength than actually exists. This definition of conservative is used throughout the study. Caution must be exercised in directly extending the results of this study to more conventional applications. In such use the assumptions of this study would become unconservative.

CHAPTER II

LITERATURE REVIEW

2.1 General

Many papers have addressed the topic of progressive collapse of damaged structures, but only one has provided a general quantitative method of analysis (1). The following sections summarize the published papers while the last section details the one general approach.

2.2 Qualitative Analysis

Most studies of progressive collapse, as applied to structures conventionally associated with Civil Engineering, fit into three categories. The first addressed a need to predict statistically the frequency and severity of damaging events such as vehicle impact or explosion (2 through 8).

Another category was the qualitative analysis of a structure's ability to resist damage or to develop alternate load paths around damage. Typical topics of discussion included catenary action of slabs, beam action of adequately tied ceiling-wall-floor systems acting as wide flange sections, and the in-plane arching of walls over damage (4, 7, 9 through 14).

A third category was an effort to develop codes which mate the first two areas into economically and socially acceptable guidelines for design and construction (15 through 23). Additionally, a research workshop was

conducted in 1975 to evaluate present knowledge of the progressive collapse phenomenon and to identify areas requiring further study (24).

2.3 Quantitative Analysis of Building Structures

A smaller, fourth category addressed the need to evaluate quantitatively the behaviors occurring during a progressive collapse. Several studies have been completed, but most have considered only two-dimensional problems and most have required extensive analyst interactive involvement (25 through 29).

Smith and Epstein (30) developed a three-dimensional method to analyze the progressive collapse of a space truss roof. Their approach used the finite element method to determine structural member stresses. As a member approached its buckling load, predetermined for every member in the structure, the member was replaced by opposite equal forces representing post-buckling strength. The method did provide a three-dimensional analysis but was limited exclusively to buckling related failures. It was inappropriate for structures in which other failure modes share equal importance or are dominant.

2.4 Quantitative Analysis of Aircraft Structures

The military's need to predict the behavior of damaged aircraft has precipitated several papers of interest. Venkayya (31) outlined an empirical iterative procedure in 1978 for determining the residual strength of damaged structures. The displacements and decomposed stiffness matrix of an undamaged structure were combined with a sparse negative stiffness matrix representing damage. The result was an iteratively derived second-order Taylor series approximation of the response of the damaged structure.

The method appeared suitable for economic evaluation of initial structure response to several different damage conditions. However, when the method is applied to progressive collapse analyses, problems surface as component failure progresses toward collapse. Solution convergence times become unacceptably slow and convergence criteria become increasingly difficult to establish.

In 1976, Heard (1) proposed for the Air Force Armaments Testing Laboratory (AFATL) a method of structural modeling and analysis for progressive collapse in aircraft structures. That method, referred to in this study as the AFATL method, appeared to be the most promising general approach to a quantitative analysis of progressive collapse. The next section presents this method in some detail.

2.5 Analysis Method Background

The structure being evaluated must be represented as a computer model for finite element analysis. Because the method requires many iterative analyses to trace the progressive collapse phenomenon, economy urges the use of the largest, simplest elements which still describe the basic geometry of the structure and provide adequate precision to permit a stress-based analysis. A principal feature of the AFATL method is that little or no refinement of the model occurs in the area of damage. This feature aids the economy of the method but, because the large elements mask stress concentrations, the method must include compensating techniques. Heard employed two such techniques which are described later.

A load was applied to the model and the resulting stresses were examined in search of overstressed elements. An overstressed element was one whose stresses exceeded predefined limiting values. A solitary over-

stressed element was removed from the model as having failed. If more than one overstressed element occurred grouped together, only the most severely stressed element of the group was removed. This technique helped represent crack propagation in a model composed of large elements and was supported by studies of Sih and Hartranft (28).

Reducing the values of limiting stresses for elements bordering damage was the second technique to compensate for loss of stress concentrations around crack tips at the edges of damage. Thus the computed stress in an element bordering damage might produce element failure while a similarly stressed element away from damage remained intact. Using different values for limiting stresses complicated the process of selecting which element to fail in a group of overstressed elements. The most severely stressed element could not be determined through a direct comparison of the magnitudes of element stresses.

After the failed elements were removed, the modified model was again analyzed and the procedure was repeated. Iterations continued until the model could sustain some desired maximum load, or until failure occurred. This latter condition was sometimes determined subjectively by evaluating the structure's displaced shape rather than by its residual load-carrying capacity.

CHAPTER III

SPECIFIC OBJECTIVES

The method proposed by Heard appeared to be a versatile approach for the quantitative analysis of progressive collapse. To increase the acceptability of the method, however, four areas were identified as objectives for further study.

Validation of the method was perhaps the most important objective. Due to an absence of actual aircraft wings which his model represented, Heard was unable to substantiate with actual test data the value of his work. The first phase of this study was a laboratory test program which provided data for evaluation of analytical results. Tests of three F-84F aircraft wings measured structural performance under different damage and load combinations.

In the previous study, only one combination of elements was used for modeling the aircraft wing structure. A comparison of several element combinations was made in search of the best selection of model elements.

The actual application of the method required a great amount of manual data analysis for each iteration. A large number of elements had to be checked and compared to limiting stress values. The relative locations of overstressed elements had to be determined and caution applied to remove the appropriate element. Finally, removal of failed elements required modifications of the model. In addition to removing the failed elements, modification included reducing limiting stresses for elements

bordering the newly propagated damage. Automating the application of the method was desirable to reduce both time and expense for a complete analysis.

The final area to address was the appropriate values for limiting stresses. Heard used two levels of limiting stresses: material ultimate strength for elements away from damage, and material yield strength for elements bordering damage. A more sophisticated determination of limiting stresses had the potential for returning more realistic results.

These four areas,

1. Comparison of analytical and test results
2. Comparison of modeling elements
3. Automation of the method
4. Determination of limiting stress values,

became the specific objectives of this study.

CHAPTER IV

EXPERIMENTAL TEST PROGRAM

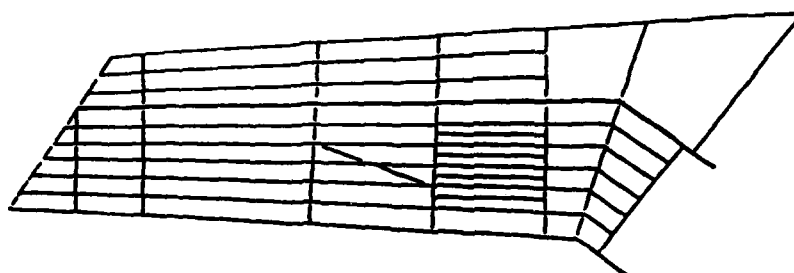
4.1 General

A major objective of this study was to use actual test data as a standard for evaluating analytical results. Three F-84F aircraft wings were tested, each with a different damage and load combination. This chapter contains descriptions of specific damage and loads and of the general test procedure. Appendix A contains diagrams showing strain gage locations for the various tests.

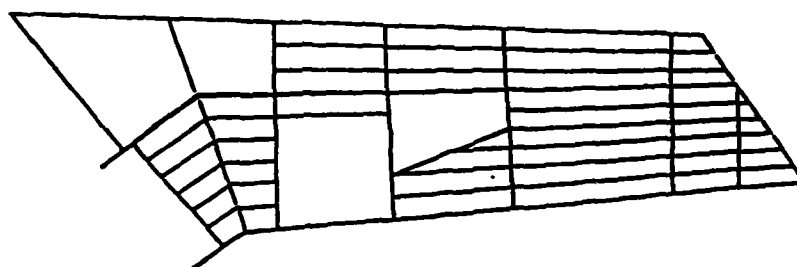
4.2 Specific Test Descriptions

The F-84F aircraft wing is a two-spar, semi-monocoque structure. For general reference, Figure 1 illustrates the upper and lower wing surfaces and the wing structural frame. All three wings were mounted upside down for testing; however, the terms "upper" and "lower" refer to the wing's upper and lower surfaces, not to their physical orientation for the tests. For all tests, the landing gear and gear doors, flaps, and ailerons were removed.

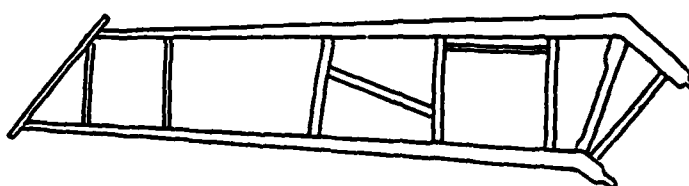
Test 1 consisted of severe damage to the upper half of the front spar as shown in Figure 2. A load applied to the front spar produced a failure with bending as the predominant behavior. The damaged area was stressed in tension.



(a) Upper Surface

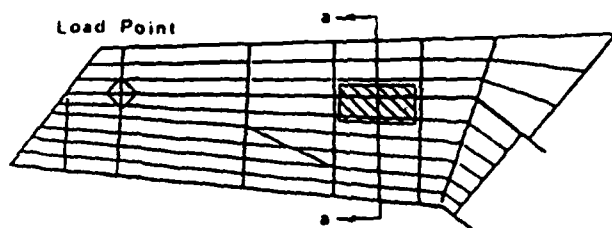


(b) Lower Surface

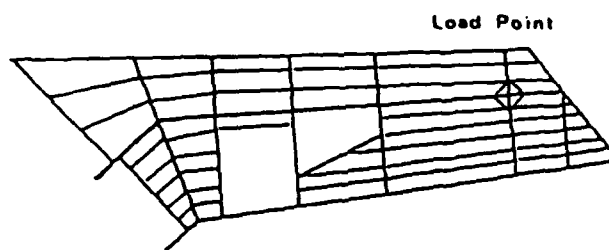


(c) Structural Frame

Figure 1. F-84F Wing Structure



Upper Surface



Lower Surface



Section a-a

 Damage

Figure 2. Damage for Test 1

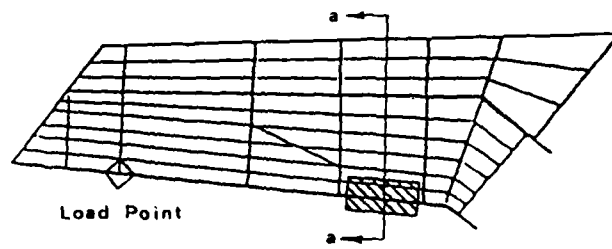
Test 2, Figure 3, was to measure behavior with a significant amount of torsion present. The rear spar was completely severed and the load was applied to the rear spar. The result was a combination of bending and torsion in the front spar. This wing could not be failed within safe limits of the laboratory loading apparatus. Consequently, three loading trials were performed on this wing and designated Tests 2A, 2B, and 2C. Each test had slightly modified damage to the skin adjacent to the severed spar. These were efforts to initiate tearing of the skin over the wheel well area; however, no propagation of that damage occurred.

Test 3 was an attempt to represent more closely the damage which could occur from a shaped-charge missile warhead. Figure 4 shows a 5½-inch wide strip of material removed from the lower wing surface. All skin was removed from the strip, which extended from the rear spar to the leading edge. The lower rear spar cap was removed but the web was left intact. The portion of the lower front spar cap extending from the web toward the trailing edge was also removed. The load applied to the rear spar put the damaged surface into compression.

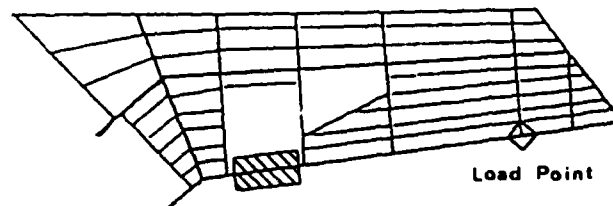
The residual strength of this wing also exceeded the safe capacity of the loading equipment. A variation of this test, designated Test 3B, included further damage to the front spar cap. Half the width of the lower spar cap extending from the web toward the leading edge was removed. A 1¼-inch width of spar cap remained extending from the rear face of the web toward the leading edge. This additional damage led to complete structural failure.

4.3 Wing Support System

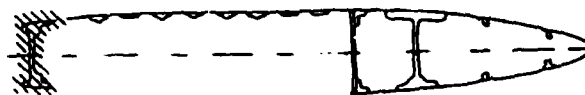
Each wing spar root mounted into a support structure as illustrated



Upper Surface



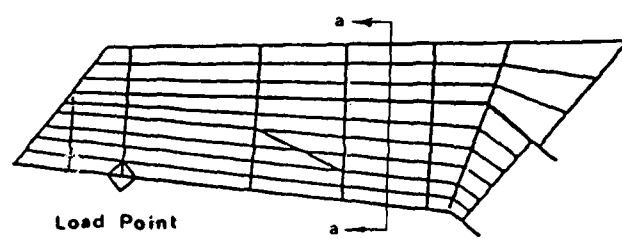
Lower Surface



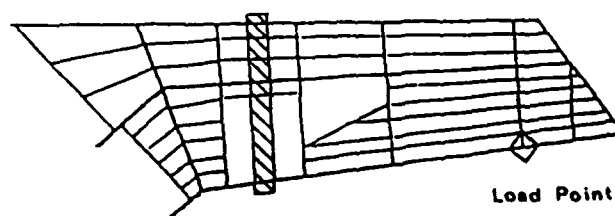
Section a-a

 Damage

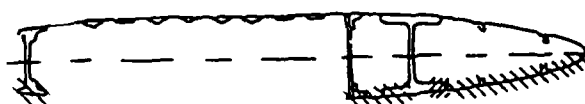
Figure 3. Damage for Test 2



Upper Surface



Lower Surface



Section a-a

-  Damage
-  Additional Damage for Test 3B

Figure 4. Damage for Test 3

in Figure 5. Pins secured the wings in the support structures in the same manner as the wings had been attached to aircraft fuselages. The support structures were extremely rigid compared to the wings so that no appreciable deformation occurred within the supports themselves. Three transducers supported each T-shaped support structure, permitting measurements of vertical reaction forces and reaction moments about two perpendicular horizontal axes.

The wing spar roots were aligned with the support structures and pinned into place within small tolerance; however, some motion of the wing spar roots with respect to the supports was unavoidable. For Tests 2 and 3, dial gages measured relative rotation of each wing spar root about horizontal axes parallel to and perpendicular to the root itself. These data then formed the basis for support conditions in corresponding finite element analyses. These support conditions provided a better analytical representation of wing deflections; however, support conditions assuming no relative rotation were used for stress analyses.

4.4 Load System

A movable overhead crane applied a single point load in each test. The crane was self-adjusting so the load was always applied vertically. A cable attaching the crane to the wing load point was equipped with an in-line transducer to permit continuous accurate monitoring of the actual load applied.

4.5 Deflection Measurements

Vertical deflections were measured at points along the front and rear spars corresponding to node points in the finite element model.

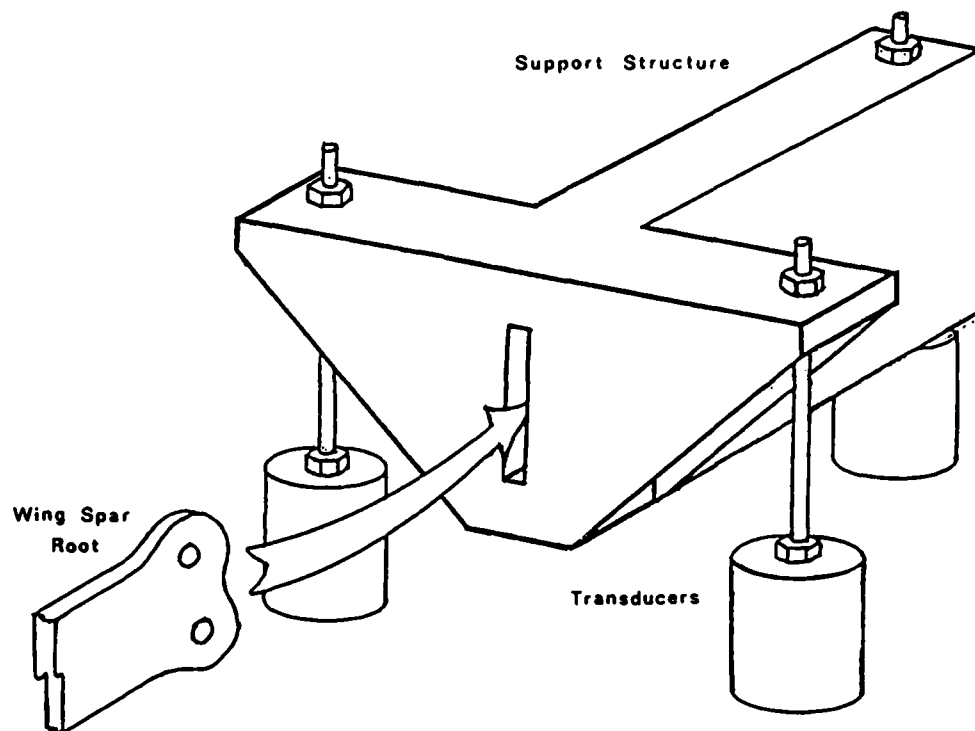


Figure 5. Wing Support Structure

Steel scales incremented to 0.01 inch were attached to the lower wing surface and measurements were read through an engineer's level. Similarly, scales were mounted on the support structures above each transducer to detect any vertical displacements there. At the load point no scale could be attached to the wing surface as at other locations along the spars. Instead, a cloth tape hung down vertically from the upper wing surface to measure deflections with respect to the laboratory floor.

4.6 Strain Measurements

Strain gages were mounted to the wing to detect changes in load paths as components failed and to detect load levels at which failures occurred. The different designs of each test and experience from previous tests led to slightly different strain gage placement for each wing. Appendix A contains specific locations.

Quarter-inch uniaxial strain gages measured outer fiber strains along spar and rib caps. Three-gage rectangular rosettes attached to selected skin panels measured panel behavior. Similar rosettes measured shear in rib and spar webs in Test 3.

Wings were first loaded enough to compensate for self-weight, and all gages were zeroed. For Test 1, all transducers and strain gages fed into a single switch and balance unit to measure output. All other tests used a Vishay Instruments Measurements Group computer-controlled data acquisition and reduction system. The System 4000 included the software program plus a Controller 4220 and two Strain Gage Scanners 4270. A Hewlett-Packard 9825B, upgraded to 9825T capabilities, served as the Executive Control Unit to complete the system hardware.

CHAPTER V

FINITE ELEMENT MODELS

5.1 Background

The fundamental modeling philosophy used by Heard (1) applied also to this study. Rod elements in combination with shear panels represented heavy structural members such as spars and ribs. Shear panel or membrane elements represented aircraft skin. Skin stiffeners were modeled by rod elements.

The specific structure for this study, the F-84F aircraft wing, was also analyzed by Jordan (32, 33). In 1976, he performed a dynamic response and small static load bending analysis of the wing. Although his objective differed from Heard's, he applied the same fundamental philosophy to develop his model of the wing. Jordan's model was the nucleus of the models evaluated in this study and is illustrated in Figure 6. Details of element numbering are presented in Appendix B.

The structure's geometry determined the size of the elements. Intersections of spars and ribs and of skin stiffeners and ribs were model node points. The node points in turn defined the elements. The procedure for assigning area properties for elements, particularly for rods, was detailed by both Heard (1) and Jordan (33). A brief summary is presented in Appendix C.

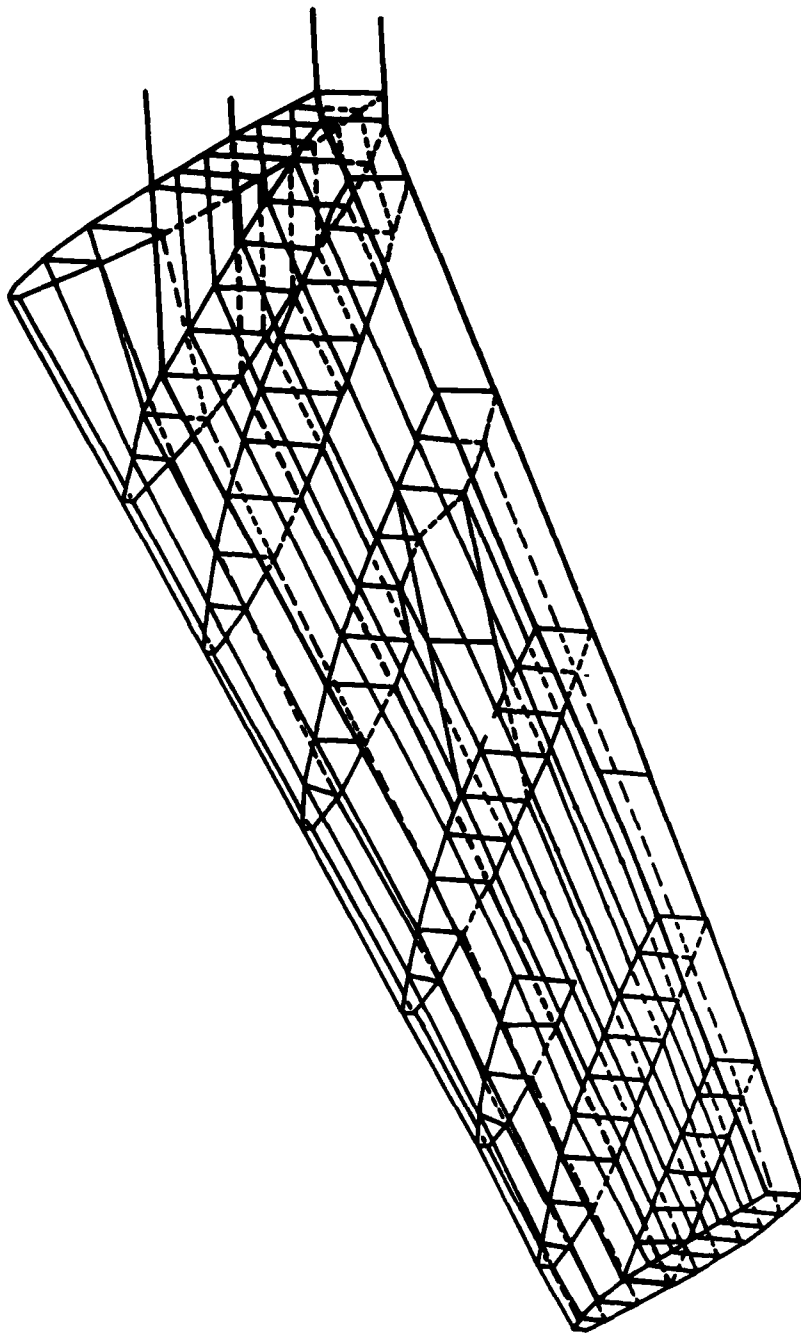


Figure 6. F-84F Finite Element Model

5.2 Model Variations

The model developed by Jordan gave him good response for conditions where bending dominated; however, it provided no torsional stiffness for the heavy structural members. To evaluate damage and load combinations producing significant torsion, model revisions included torsional stiffness for the front and rear spars.

This stiffness was provided by including rod elements along the centerlines of the spars. These elements had no axial load capacity but did provide torsional resistance. Multipoint constraint equations determined the rotation of each end of a torsion rod by using the lateral displacements of the nodes immediately above and below it. Figure 7 illustrates that the rotation, β , of the end of the centerline rod was

$$\beta = \frac{1}{h} (y_u - y_l) \quad (5.1)$$

Although modeling philosophies in the previous efforts were essentially the same, Heard used membrane elements for the skin while Jordan used shear panels. This study compared four modeling combinations. All four models used rods for skin stiffeners and for caps of spars and ribs. All used shear panels for spar and rib webs. The differences are presented in Table I. Appendix D is a listing of Model A and Appendix E is a listing of Model C. The additions for torsional resistance to convert Models A and C to Models B and D, respectively, are presented in Appendix F.

5.3 Modeling Initial Damage

To the extent possible, no special modeling techniques were applied to initial damage. Damaged shear panels or membranes were reduced in

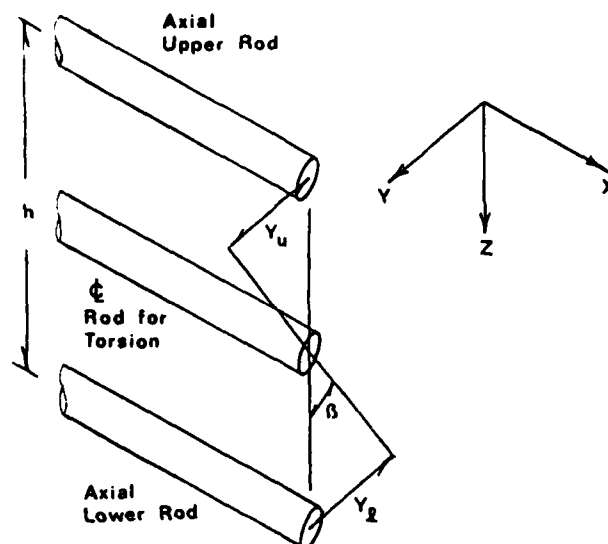
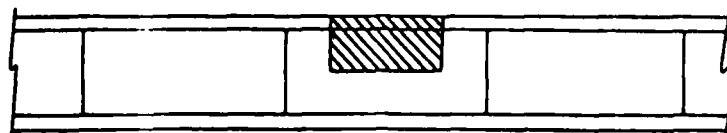


Figure 7. Rod Elements for Spar Torsional Capacity

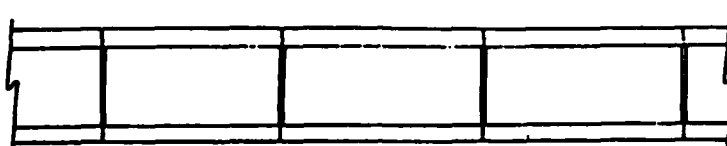
TABLE I
DESIGNATION OF FINITE ELEMENT MODELS

Model Designation	Skin Elements	Torsional Stiffness
A	Shear Panels	No
B	Shear Panels	Yes
C	Membranes	No
D	Membranes	Yes

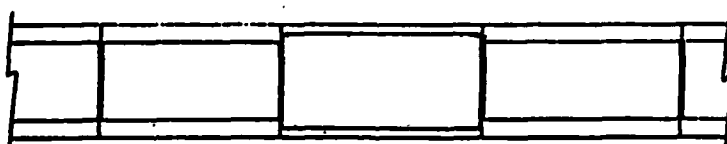
thickness or were removed when damage was severe. The rods representing damaged spars were reduced in size to maintain an equivalent moment of inertia, as presented by Jordan (32, 33). One exception to this approach was evaluated for Test 1. Damage to the front spar extended halfway into the web. Figure 8a shows a side view of the damaged front spar and Figure 8b shows modeling of the undamaged spar. The simpler modeling technique is illustrated in Figure 8c. The web element thickness was reduced to half its undamaged size. Rods representing spar caps were unmoved but reduced in size to represent the residual moment of inertia. Figure 8d shows the more detailed approach used by Jordan (32, 33) to model such severe damage. The two methods were compared. The specific changes made to Models A and C for each test are presented in Appendix G. The additional changes for Models B and D are included as part of Appendix F.



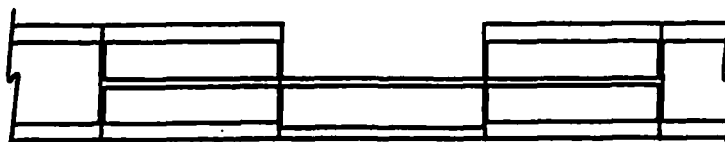
(a) Front Spar. Damaged



(b) Model. Undamaged



(c) Model. Simple Damage



(d) Model. Detailed Damage

Figure 8. Modeling Variations for Test 1 Damage

CHAPTER VI

ANALYSIS AUTOMATION

6.1 General

One shortcoming of the AFATL method cited in Chapter III was the need to examine voluminous computer output. A FORTRAN IV computer code, entitled PROSCAN for Progressive Structural Collapse Analysis, was written to alleviate the problem. PROSCAN was written to apply the method in conjunction with NASTRAN (National Aeronautics and Space Administration Structural Analysis) to perform the finite element analyses. Figure 9 illustrates the analysis procedure, and Appendix H comprises a functional flow chart and a listing of the PROSCAN program.

In exchanging information between the two computer programs, disk storage was used exclusively. All NASTRAN output was stored in punched-card format in disk files. All case control and bulk data decks were also stored on disks, and all modifications made by PROSCAN to the models were directed to those storage files.

6.2 Overstressed Elements

The first requirement in applying the AFATL method to finite element results was identifying overstressed elements. Because more than one limiting stress value was permissible, some common basis for evaluating severity of stress had to be established. The criterion selected was the margin of safety defined as

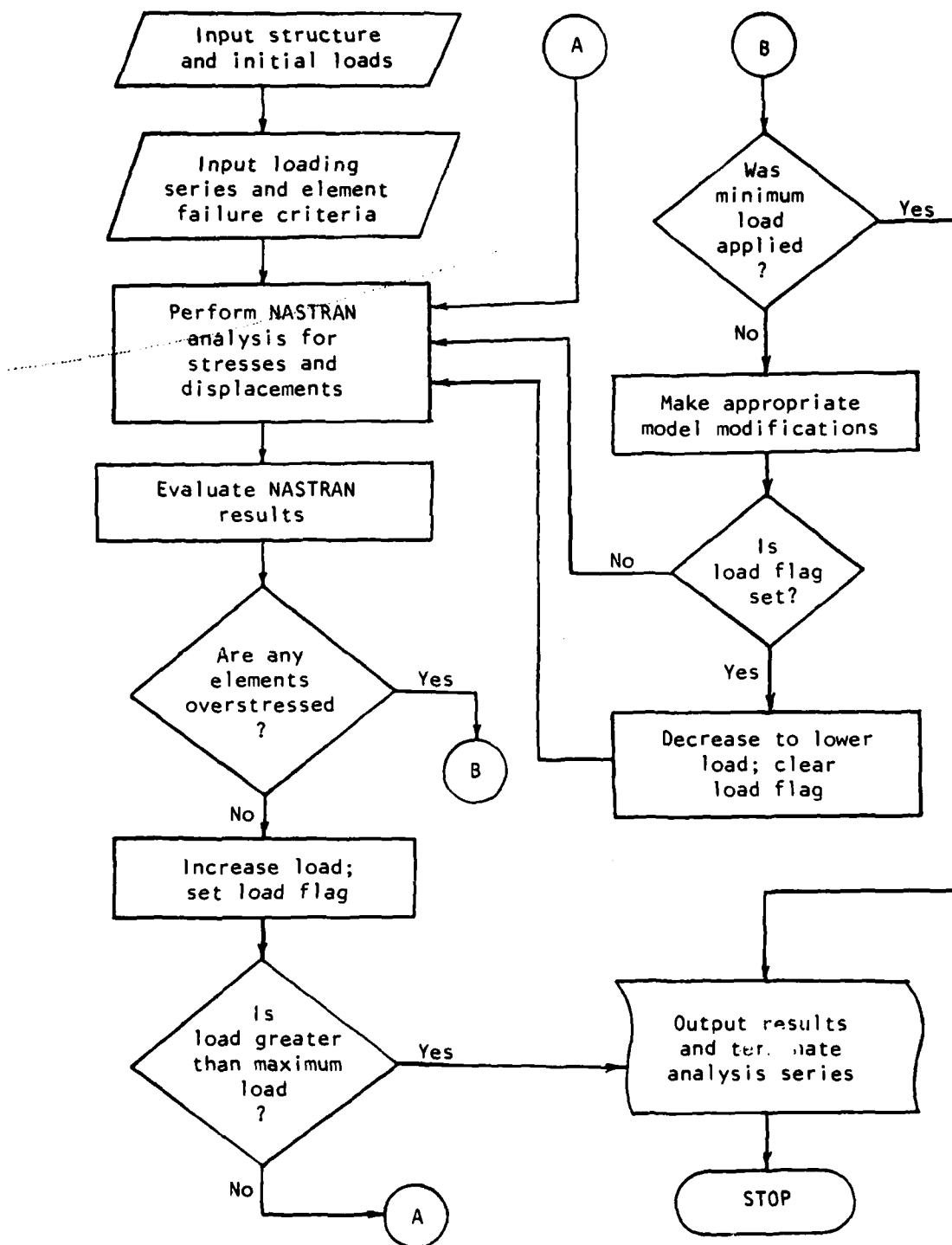


Figure 9. Iterative Analysis Procedure

$$M.S. = \frac{\text{allowable stress}}{\text{actual stress}} - 1.0 \quad (6.1)$$

Several elements within NASTRAN, rods and shear panels included, return an element margin of safety as part of the solution. For those elements which do not provide a margin of safety, PROSCAN calculated one. Element principal and maximum shear stresses were compared to analyst-provided limiting stresses for tension, compression, and shear. A margin of safety was calculated for each type of stress and the algebraically smallest value was selected as the element margin of safety. PROSCAN then identified any element with a negative margin of safety as an overstressed element.

6.3 Failed Elements

PROSCAN applied the next step of the AFATL method, grouping of overstressed elements, by node matching. The node numbers of each overstressed element were compared to those of every other overstressed element. PROSCAN designated any continuous linkage of those elements as a group, then selected the most severely stressed element from the group.

The element margin of safety again was the basis for decisions. PROSCAN selected the element of the group with the most negative margin of safety. That element became a failed element. The process of grouping and failing elements continued until all overstressed elements were considered.

PROSCAN did not actually remove a failed element from the model. Instead, PROSCAN assigned property values to the element which effectively eliminated its contribution to the structure. The failed areas and moduli of elasticity and shear were orders of magnitude below nominal values for unfailed elements.

6.4 Propagation of Damage

The failing of an element represented propagation of the damage, and as a consequence, the borders of the damage expanded. Additional elements had to be identified as bordering the new damage so they could be assigned reduced limiting stresses. Again a node matching scheme was employed. Each element which had at least one node in common with a newly failed element was examined. If it had not already failed itself or had not already bordered damage, lower limiting stresses replaced those previously used. The lowering of limiting stresses accounted for the possible presence of crack tip stress concentrations as introduced in section 2.5.

6.5 Adjustment of Load

PROSCAN had the capability of applying a new load to the model with each iteration. That capability was used in this study as explained below.

If no element failed on a particular iteration, the load was increased for the next NASTRAN analysis. This would occur until the structure sustained some maximum user-specified load without further element failure. Conversely, if an element failed on a particular iteration, PROSCAN reduced the load for the next NASTRAN analysis. The purpose was to determine the structure's ability to carry a lesser load after further weakening by the failed element. Reducing the load every time an element failed continued until the structure could not sustain a minimum load without further failure.

The analyst provided a sequence of loads to be applied, from minimum to maximum, as part of the PROSCAN input data. PROSCAN then made the

appropriate changes to the NASTRAN case control deck to reflect the structure's performance on the previous iteration. In addition to changing the load identification number, PROSCAN could assign new single point and multipoint constraint sets and identify new labels to correspond to each new load.

PROSCAN automated the entire application of the AFATL method. This began with initial viewing of NASTRAN output and finished by establishing new files containing modified case control and bulk data decks.

CHAPTER VII

DETERMINATION OF LIMITING STRESSES

7.1 Need for Limiting Stresses

Repeated reference has been made to limiting stresses. It is appropriate to address in more detail the specifics of allowable stress levels. Heard (1) used two limiting stress criteria: ultimate strength for elements away from damage, and yield strength for elements bordering damage. This study attempted to define more precisely the levels of stress which should cause failure in the model.

Ultimate strength remained the basic criterion for defining failure; but most elements, even those away from damage, were assigned limiting stresses lower than ultimate strength. Consider that a relatively large element returned a computed stress representative of a large structural region. This representative stress was unavoidably lower than the high stress within the region which would cause failure in the actual member. It was necessary then to estimate the effects of the representative stresses by using some value of limiting stress lower than ultimate strength.

Appropriate limiting stresses also estimated the nonlinear behavior experienced through the buckling of skin panels. Since the finite element analyses assumed linearly elastic behavior, the ability to compensate for skin panel buckling was incorporated to enhance results. PROSCAN had the ability to incorporate both the low stresses causing buckling and the reduced stiffnesses subsequent to buckling.

7.2 Limiting Stresses for Rod Elements

Spar and rib sections were each represented by three elements. A shear panel represented the web. One rod element represented the upper spar cap and another rod represented the lower spar cap. The rods were sized and spaced to maintain the moment of inertia about the section's neutral axis and to return outer fiber stresses.

The stress value obtained for rod elements was the average of the stresses at each end of the rod. Because rod elements in the spars were relatively long, the average stress could be substantially less than the maximum stress. A procedure to obtain limiting stresses for a similarly modeled doubly symmetric cantilevered beam served as a foundation for developing limiting stresses for the wing model. Figure 10 shows such a beam with top rod elements numbered and top nodes lettered.

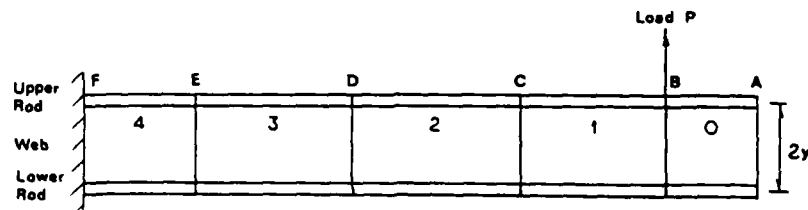


Figure 10. Cantilevered Beam Model

Using rod 3 for illustration, the maximum stress from the applied load occurred at node E, but the stress obtained was the average of

stresses at nodes D and E. Designating L_j as the length of any member j , the average stress in rod 3 was

$$\sigma_{\text{avg}} = \frac{P(L_1 + L_2 + \frac{1}{2} L_3)y}{I} \quad (7.1)$$

and the stress at node E was

$$\sigma_{\text{max}} = \frac{P(L_1 + L_2 + L_3)y}{I} \quad (7.2)$$

where I was the section moment of inertia. Designating a limiting stress factor, F_i , as the ratio of σ_{avg} to σ_{max} ,

$$F_3 = \frac{L_1 + L_2 + \frac{1}{2} L_3}{L_1 + L_2 + L_3} \quad (7.3)$$

In general terms,

$$F_i = \frac{\frac{1}{2} L_i + \sum_{j=1}^{i-1} L_j}{\sum_{j=1}^i L_j} \quad (7.4)$$

for the single point load shown. The appropriate limiting stress, σ_{L_i} , was

$$\sigma_{L_i} = F_i \sigma_{\text{ult}_i} \quad (7.5)$$

where σ_{ult_i} was the ultimate strength for the member i .

To extend this approach for calculating stress factors to the finite element model of the wing, the wing itself was idealized as a straight cantilevered beam. The front spar dimensions were used for section lengths as shown in Figure 11. Upper surface element numbers are below each rod and corresponding node numbers are above each node.

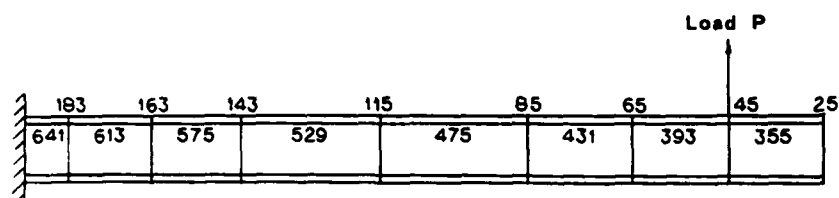


Figure 11. Cantilevered Front Spar Idealization

Factors to reduce material ultimate strength for elements inboard of the load were calculated as shown in the previous example. Elements between the load and the wing tip used the same factor as the elements immediately inboard of the load. Table II shows the limiting stress factors for the front spar rods on the upper wing surface.

TABLE II
LIMITING STRESS FACTORS FOR ROD ELEMENTS

Rod No.:	641	613	575	529	475	431	393	355
F_i :	0.950	0.937	0.921	0.855	0.788	0.735	0.500	0.500

Each rod representing a skin stiffener was approximately parallel to the spars and was assigned the same factor as its corresponding spar element. Each rib was approximately perpendicular to the spars. Each rod in a rib was assigned the factor of the spar rod immediately inboard of the spar-rib intersection.

A single concentrated load was used for analysis to correspond to the actual loading applied in the laboratory test program. However, an aerodynamic load could be represented by any approximation acceptable to the analyst. Although the mathematical expression for F_i would be more complex, the same approach to factoring for limiting stresses in rod elements could be applied.

7.3 Limiting Stresses for Web Elements

Shear panel elements represented the webs of spars and ribs. The limiting stress for shear was determined by comparing the average shearing stress in the web to the maximum shearing stress in the web. If V were designated as the shearing force in the cantilevered beam discussed in the previous section, the web element yielded a shearing stress of

$$\tau_{avg} = \frac{V}{2yt} \quad (7.6)$$

where t was the web thickness. The maximum shearing stress in the section was

$$\tau_{max} = \frac{VQ}{It} \quad (7.7)$$

The limiting stress factor, F , was the value of τ_{avg} divided by τ_{max} , so the limiting shearing stress, τ_L , was

$$\tau_L = F \tau_{ult} \quad (7.8)$$

Calculations for typical spar cross sections showed $F = 0.35$ to be a representative value. This value was applied to all spar and rib web elements.

Proportions of spar and rib sections indicated web crippling was unlikely; therefore, no reductions in limiting stresses were developed for buckling of undamaged web members. If initial damage to the structure introduced a potential for buckling, residual member proportions dictated the appropriate reductions.

7.4 Limiting Stresses for Skin Elements

Skin panels, unlike spar and rib webs, were susceptible to buckling. Additionally, the skin could tear along rivet lines or rivets themselves could fail. Whether a panel buckled in shear or in compression, or failed along a rivet line, the result was a reduction in stiffness of the panel. Because of the similar change in behavior, panels were divided into either pre-buckling or post-buckling categories even though rivet line failure was not a buckling phenomenon.

Each skin panel on the wing had slightly different geometric properties which gave each slightly different pre- and post-buckling characteristics. Panel 106, forward of the front spar on the lower wing surface, was typical of most skin panels and was used to determine approximate values for all panels. Figure 12 shows its location in the model. The assembly used for calculations included panel 106, a stiffener attached to each long side, and a rib attached to each short side. Averaging the lengths of the two long sides and the two short sides gave a rectangular shape for calculations.

To determine pre-buckling limits, compression perpendicular to the long sides, compression perpendicular to the short sides, and a corner force producing shear were all evaluated separately. Calculations were determined according to Peery (34, Chapters 14 and 15). The average

stress causing buckling in each case was divided by the material ultimate strength to determine the limiting stress factors, F . For the two compression conditions, the more conservative value was used.

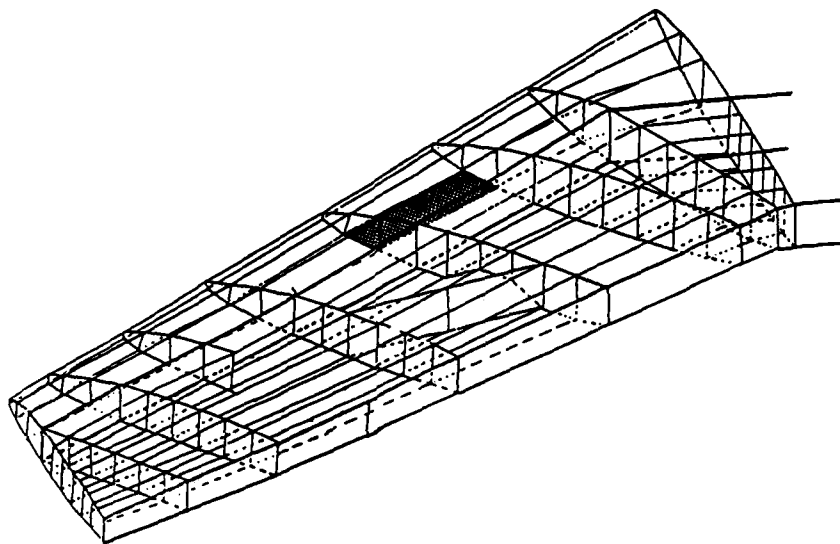


Figure 12. Typical Panel for Buckling Limits

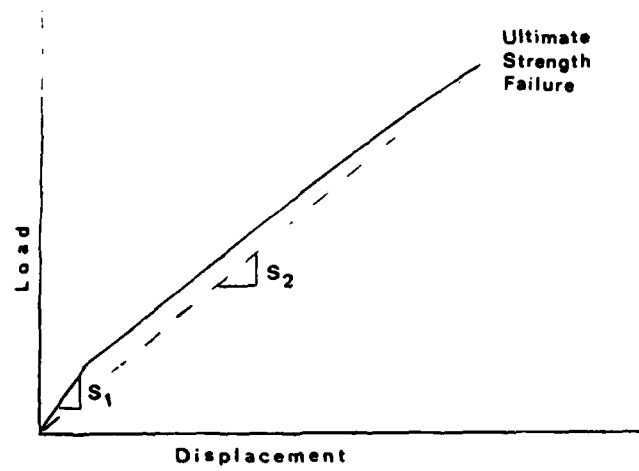
Limits for tension were obtained by calculating rivet and skin strengths along a conservative rivet line. Limiting loads, determined according to Peery (34, Chapter 12) and Bruhn (35, Chapter D1), were divided by the ultimate load, the load causing an average stress in the panel equal to the ultimate strength. The result was the limiting stress factor. Skin failure was compared to rivet failure, and the more conservative value was used.

For post-buckling behavior, the limiting stresses were returned to ultimate strength, but the elastic and shear moduli were reduced to

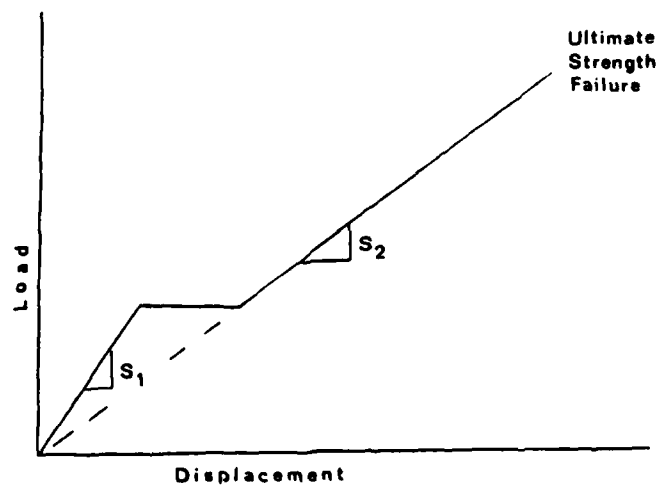
account for the reduction in stiffness following buckling. For compression buckling, bilinear behavior was assumed. The load versus displacement curve of Figure 13a assumed linear behavior prior to buckling, then linear behavior from buckling to an ultimate strength failure. The slope of the pre-buckling portion of the curve, S_1 , was divided into a secant slope, S_2 , from the origin to failure. The result was a reduction factor, M , for the elastic modulus. The same process applied to Figure 13b produced a reduction factor for the modulus of shear. Table III summarizes the results for skin buckling.

TABLE III
LIMITING FACTORS FOR SKIN ELEMENTS

Behavior	Stress Factor F	Modulus Factor M
Compression, Pre-buckling	0.17	1.00
Shear, Pre-buckling	0.32	1.00
Tension, Pre-buckling	0.55	1.00
Compression, Post-buckling	1.00	0.76
Shear, Post-buckling	1.00	0.53
Tension, Post-buckling	1.00	0.76



(a) Compression



(b) Shear

Figure 13. Load-Displacement Curves
for Panel Buckling

Although the assumption of linear behavior from buckling to ultimate failure was not correct, it was a conservative representation of the rather brittle material behavior observed in the laboratory. The result economically approximated the loss in stiffness suffered by the structure from skin buckling and rivet line failure.

7.5 Damage Propagation

No attempt was made to model ragged edges around initial damage nor to reduce element size in areas of propagating cracks. The large elements then tended to mask the stress concentrations around cracks and produced a model significantly more resistant to progressive collapse than the structure being represented.

Conventionally, the nominal stress in a cracked member would have been multiplied by a stress concentration factor, K . Its value would have been larger than 1.0 and based upon crack length and crack tip severity. The increased value for stress at the crack tip would then have been compared to an allowable stress for the member. PROSCAN used an inverse approach. Rather than increase the nominal stress returned by an element, the allowable stress was decreased by a factor F , where F essentially was the inverse of K . This further reduction of limiting stresses compensated for the absence of increased modeling detail around damage.

Any cracks occurring were assumed to originate at the initial damage or in subsequently failed elements. The further reductions in limiting stresses applied therefore only to unfailed elements bordering either initial damage or failed elements.

Separate reduction factors were determined for tension and for shear. Because cracks were assumed not to propagate in compression, no further

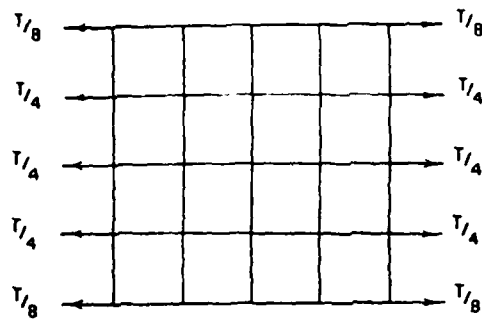
reduction applied to limiting compressive stresses. This portion of the investigation was patterned after similar crack propagation studies by Sih and Hartranft (28).

Two square plate models, one loaded in tension and the other in shear, provided information for reduction factors. Model detail ranged from two elements along a side to thirty-two elements along a side.

A crack initiated at the center of one edge propagated through the plate during sequential analyses. Loads remained constant through all iterations. Crack propagation was represented by creating a new node beside the tip of the crack, thus extending the crack to the next node. Figure 14 illustrates the procedure, exaggerated in scale, on a model using four elements per side. Figure 14b shows node m at the tip of the crack. The creation of node z extended the crack tip to node n in Figure 14c.

Figure 14b shows the plate cracked one-quarter of the way through its width. Stresses in the four elements connected to the node at the crack tip, those indicated by X's, were averaged and then divided into the average stress in the uncracked plate. The result was the limiting stress factor for the plate cracked through one-quarter of its width. The same procedure applied to the plate in Figure 14c produced the factor for the plate cracked halfway through its width. Figure 15 shows variation of the factor as a function of model detail and crack length. F_T represents tension loading and F_S represents shear loading.

All curves in Figure 15 appeared to approach zero slope as element size reduced. The values of F_T and F_S selected for this study were for a plate cracked one-eighth of the way through its width in a model with 32 elements along a side. For most components of the F-84F wing, this



(a) Un-cracked

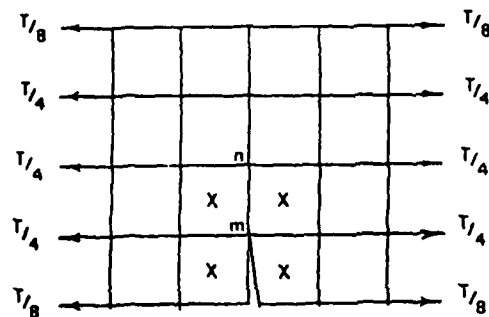
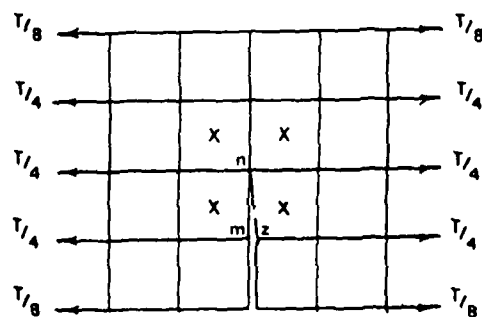
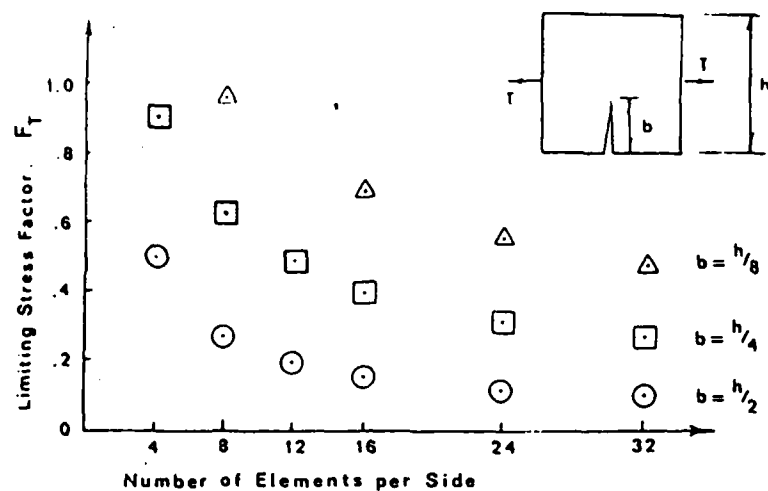
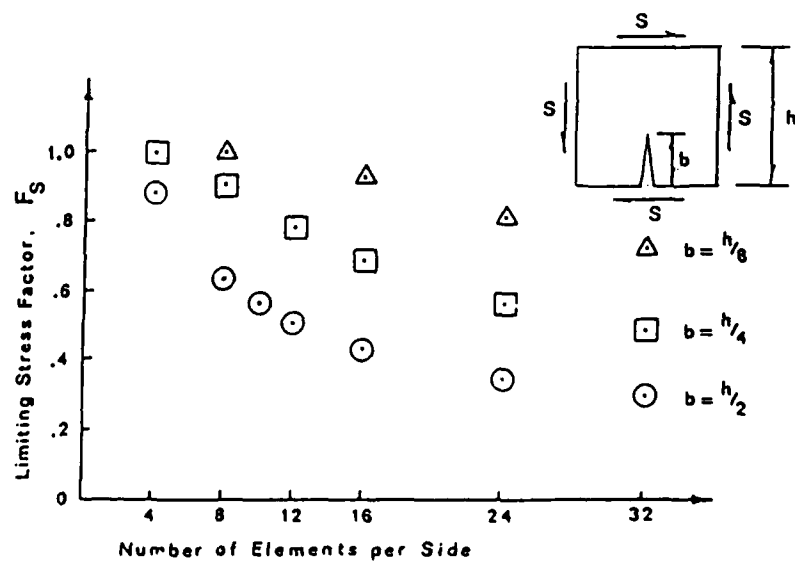
(b) Cracked Through $\frac{1}{4}$ of Width(c) Cracked Through $\frac{1}{2}$ of Width

Figure 14. Crack Investigation Models



(a) Tension



(b) Shear

Figure 15. Propagation Stress Factor Curves

represented a crack less than one inch in length in the longer side of the component. The corresponding stress concentration factor was $K=2.1$. For skin panel 106, it represented a crack 0.725 inches long with a crack tip radius of 0.30 inches calculated according to Seely and Smith (36, Chapter 12). Thus the assumed crack around damage was relatively mild and was therefore conservative.

New limiting stresses for an element bordering damage were the product of the appropriate factor, F_T or F_S or $F_C = 1.0$ for compression, and the element's previous limiting stresses. Limiting stress factors were, in this manner, cumulative. The exception was unbuckled skin panels. Their limiting stresses were not reduced to reflect cracks until after buckling stresses were exceeded.

CHAPTER VIII

COMPARISON OF RESULTS

8.1 General

Damage and load conditions for Tests 2C and 3B were analyzed using all four models for each test. Measured rotations of wing spar roots from laboratory data were enforced in the analyses. Examination of those analyses showed the addition of torsional rod elements to the spars made little difference in results. The performance of Model B was very similar to that of Model A, and the results from Model D were almost identical to those from Model C. Apparent reasons for the similarities are presented in the next section.

Further examination of the analytical results revealed unexpected stress distributions in and near the wing spar roots. The enforced rotations of wing spar roots, although developed from experimental measurements, did not produce purely rigid body motions for reasons explained in section 8.6. Consequently, the original analytical representations did not match closely enough the laboratory conditions of the experimental test program.

A second set of analyses was performed using zero support rotations for stress determination and enforced rotations for checking displacements. Tests 2C and 3B were analyzed using Models A and C for each test. Model C described the collapse phenomenon more closely than Model A as explained in section 8.4. Therefore, Model C was next compared to Model

D, the same model with the addition of torsional rod elements. The lack of significant difference between Models C and D confirmed the minimal influence of the torsional rod elements. Model B, therefore, was not analyzed further because it would produce essentially the same results as Model A. Even though torsional rod elements were not significantly affecting results, Model D was selected for the comparison of initial damage modeling since its torsional capability could provide greater latitude for an analyst to adjust model stiffness.

Model D was used to evaluate the two approaches to modeling damage for Test 1 described in section 5.3. The simpler method of modeling portrayed more accurately the pattern of failure as explained in section 8.5. The simpler method of modeling the damage was then applied to Model A for a final analysis of Test 1.

8.2 Comparison of Failure Loads

A close correlation of analytically predicted failure loads with experimentally measured failure loads would be a desirable result of evaluating the AFATL method. Table IV summarizes the failure load results. The models ranged from 5 percent to 85 percent stronger than the actual structure. Note that for Test 2 no experimental failure load was determined; therefore, conclusions about Test 2 are judgmental.

Model A gave the closest approximation for Test 3 and may have given a close approximation for Test 2. However, for reasons discussed in section 8.4, Model A was not considered the best model. Model D was more conservative than Model A in estimating wing strength. Although Model D's predicted strength for Test 2 was clearly less conservative than for Test 3, the results may have been acceptably consistent. Both approaches

for modeling Test 1 damage gave excessive predicted strengths; however, section 8.5 discusses how those figures might be improved.

TABLE IV
SUMMARY OF FAILURE LOADS

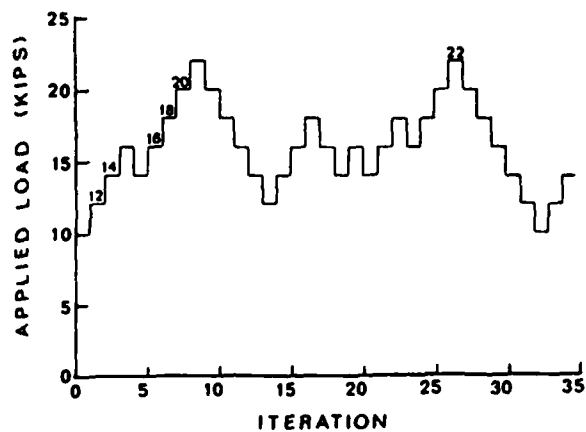
Test	Failure Loads (kips)			
	Laboratory	Model A	Model C	Model D
1	12.0	22	---	22
2	15.0*	18	20	20
3	12.4	13	19	19

* Largest load applied; no failure load determined.

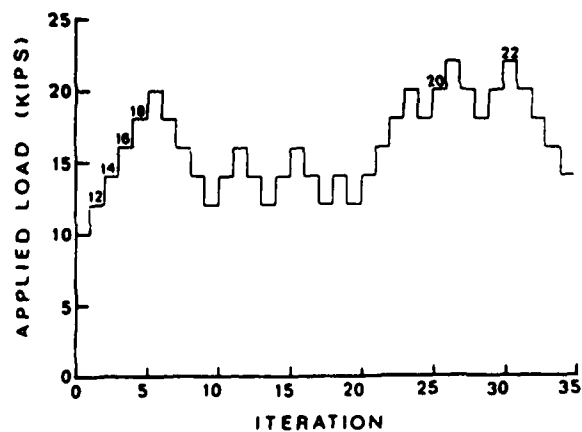
Models C and D showed no differences in failure loads and very little difference in the sequences of element failure. There are two apparent reasons for the similarity. The first is that the torsional capacities of the spars were probably underestimated when the torsional rod elements were sized. Second, bending was the dominant behavior of the F-84F wing even under extreme conditions such as those of Test 2.

8.3 Load-Iteration History

The AFATL method, as applied by PROSCAN, caused loads to vary from iteration to iteration. Figure 16 depicts the variation of load with respect to iteration for the first 35 cycles for Models A and D. The analytical data for any given load level were taken from the last cycle in which that load was applied before the model experienced a higher load. For

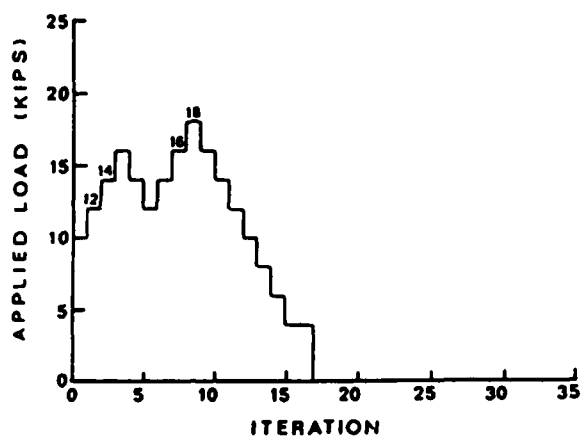


(a) Test 1, Model D (with torsional stiffness rods), Simple

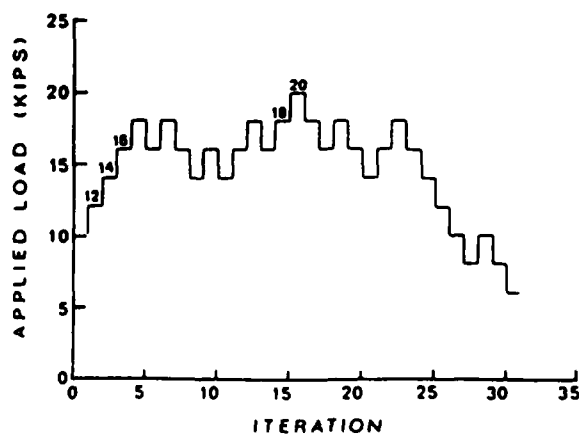


(b) Test 1, Model D (with torsional stiffness rods), Detailed

Figure 16. Load-Iteration Histories

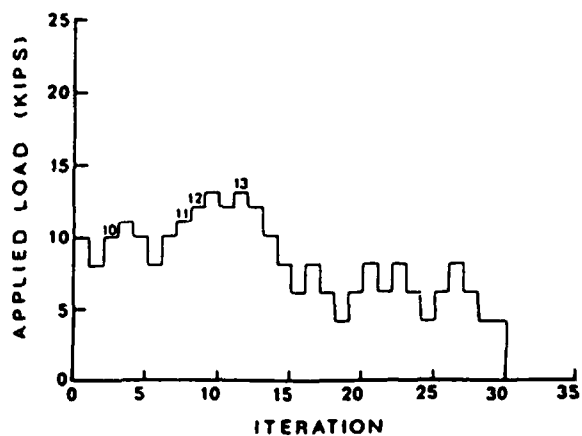


(c) Test 2C, Model A (without torsional stiffness rods)

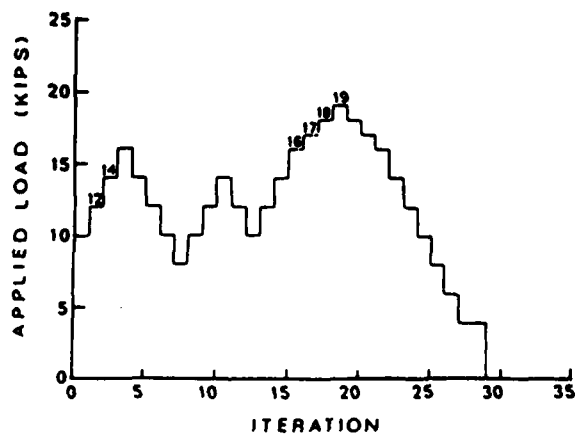


(d) Test 2C, Model D (with torsional stiffness rods)

Figure 16. (Continued)



(e) Test 3B. Model A (without torsional stiffness rods)



(f) Test 3B. Model D (with torsional stiffness rods)

Figure 16. (Continued)

example, the analytical data for the Model A analysis of Test 2C at 16 kips applied load came from iteration No. 8. As shown in Figure 16c, that was not the first application of a 16-kip load, but it was the last iteration before a higher load, 18 kips, was applied.

That procedure for selecting which iterations to use for data comparison occasionally led to gaps of several iterations between successive data-producing loads. Again as an example, Figure 16f shows 13 iterations elapsed between the 14-kip and 16-kip loads for the Model D analysis of Test 3B. During those cycles, six elements failed. This characteristic of the procedure accounted for the occasional sharp discontinuities in the plots of data.

Figure 16 also emphasizes the need for caution in setting the minimum load to be investigated. PROSCAN permitted the load to drop considerably during a series of element failures, then again rise to a high level. Figure 16a shows how the load dropped from 22 kips down to 12 kips before again climbing back up to 22 kips. Making the minimum allowable load too large could result in a premature indication of structural failure. It could occur during such a series of element failures when, in fact, the structure still possessed the capacity for loads well above the minimum level.

8.4 Internal Load Paths

The most demanding test of the models was how realistically they transferred the loads internally through the wing structure and into the supports. Figure 26 (Appendix I) compares the vertical support reactions for experimental and analytical results. Figures 27 through 29 (also

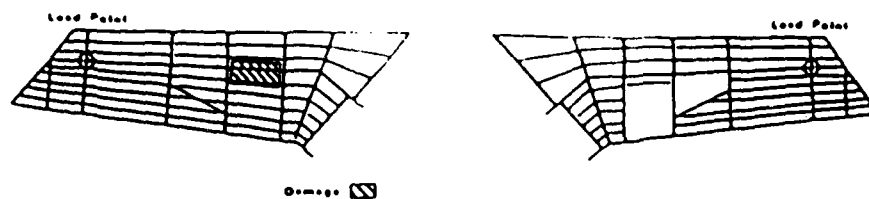
Appendix I) compare variations of strain at representative points on the wing with respect to applied load.

Examination of Figure 26 through 29 showed that neither Model A nor Model D transferred the load from the loaded spar to the unloaded spar as quickly as the actual wing did. Additionally, neither model transferred as much of the load from spar to spar as the wing did.

The most important indication for this study of how realistically the models transferred the loads internally came from Figures 17 through 19. They depict the buckled and failed elements in Models A, C, and D at their respective failure loads. For Test 3B, Figure 19, Model A did not indicate the nature of the failure as observed in the experimental test program; however, Models C and D did match closely the laboratory observations. For Test 2C, Figure 18, no failure occurred in the experimental program, but Models C and D predicted a plausible failure. Model A, however, predicted failure of the front spar at one of its strongest sections. For Test 1, Figure 17, Model D matched the laboratory failure pattern very closely using the simple modeling of initial damage. Model A, however, indicated failure of the undamaged rear spar. All models indicated more overstressing of skin elements near the wing spar roots than was observed on the actual wing. A complete summary of results for the first 35 iterations of the principal series is presented in Appendix J.

8.5 Comparison of Damage Modeling

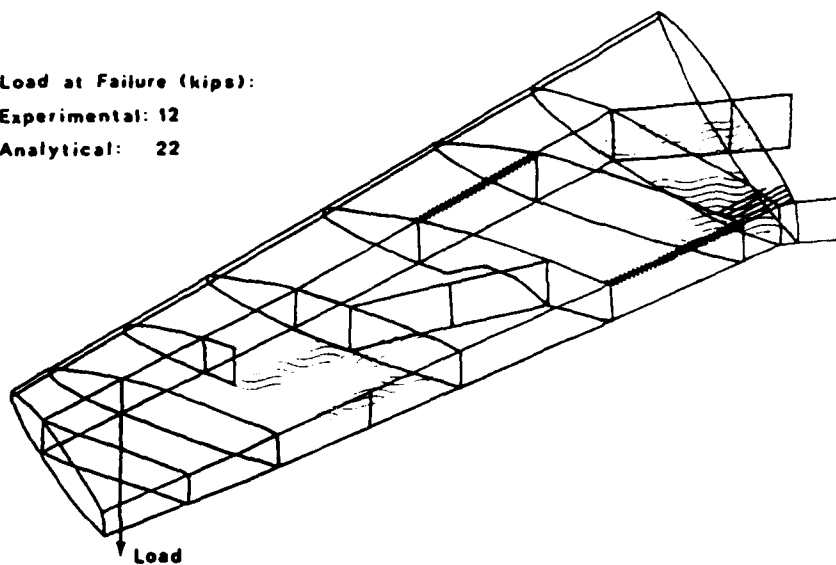
Section 5.3 introduced two approaches for modeling Test 1 damage. Both approaches predicted the same failure load, but Figure 17 illustrates that there were significant differences in which elements failed.







Load at Failure (kips):

Experimental: 12

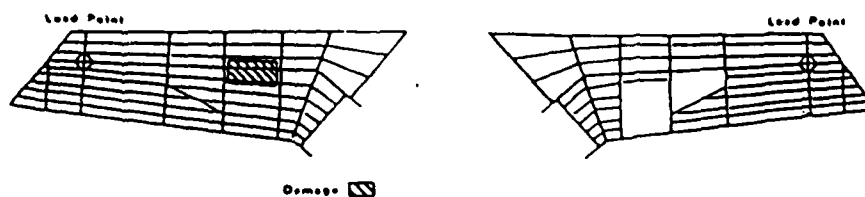
Analytical: 22



-  Buckled Shear Panel Elements on Lower Surface and Leading Edge
-  Buckled Shear Panel Elements on Upper Surface
-  Failed Shear Panel Elements
-  Failed Rod Elements

(a) Model A (without torsional stiffness rods), Simple

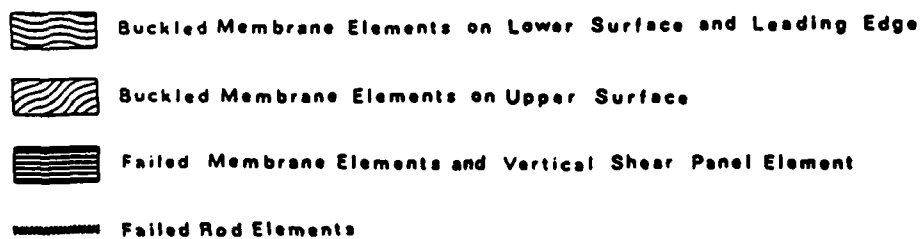
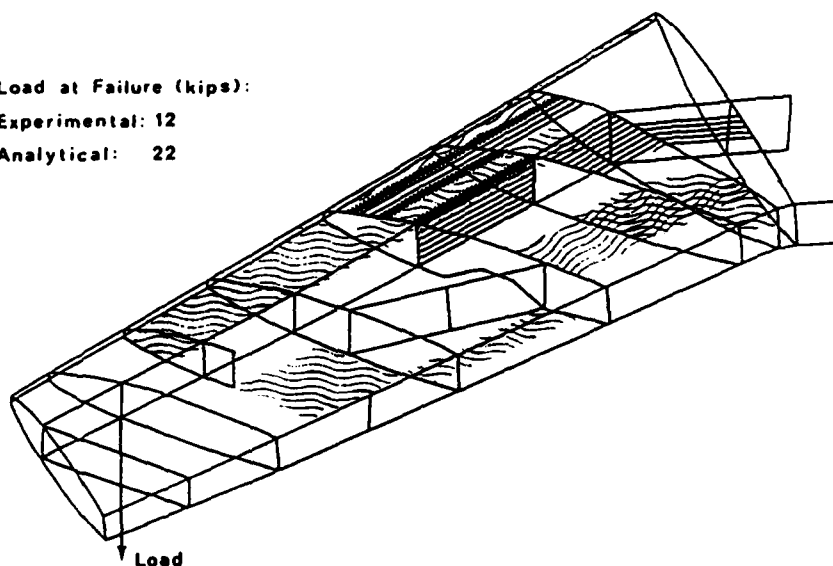
Figure 17. Wing Model Results at Test 1 Failure



Load at Failure (kips):

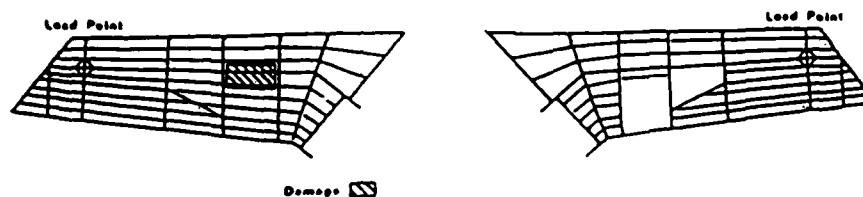
Experimental: 12

Analytical: 22



(b) Model D (with torsional stiffness rods) Simple

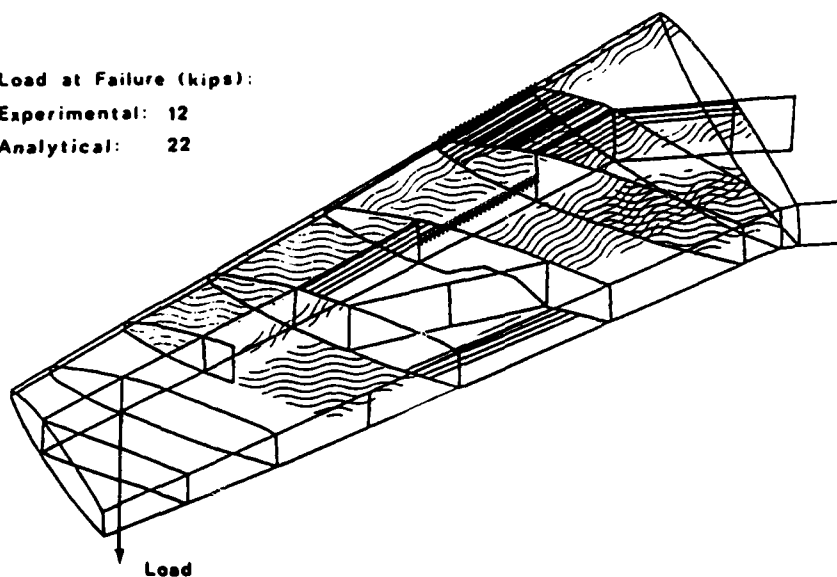
Figure 17. (Continued)







Load at Failure (kips):

Experimental: 12

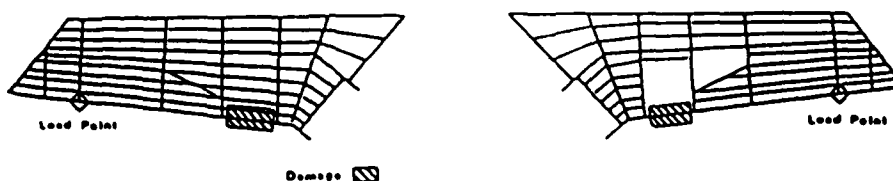
Analytical: 22



-  Buckled Membrane Elements on Lower Surface and Leading Edge
-  Buckled Membrane Elements on Upper Surface
-  Failed Membrane Elements and Vertical Shear Panel Element
-  Failed Rod Elements

(c) Model D (with torsional stiffness rods). Detailed

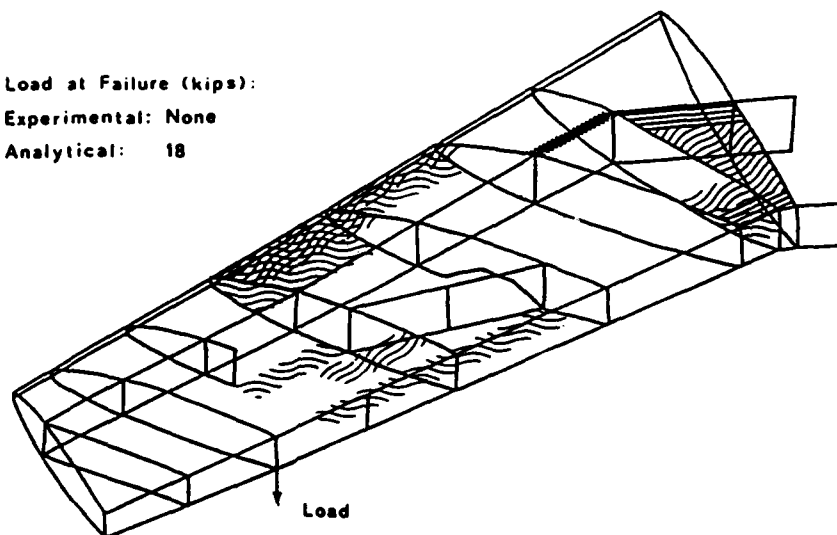
Figure 17. (Continued)







Load at Failure (kips):

Experimental: None

Analytical: 18



-  Buckled Shear Panel Elements on Lower Surface and Leading Edge
-  Buckled Shear Panel Elements on Upper Surface
-  Failed Shear Panel Elements
-  Failed Rod Elements

(a) Model A (without torsional stiffness rods)

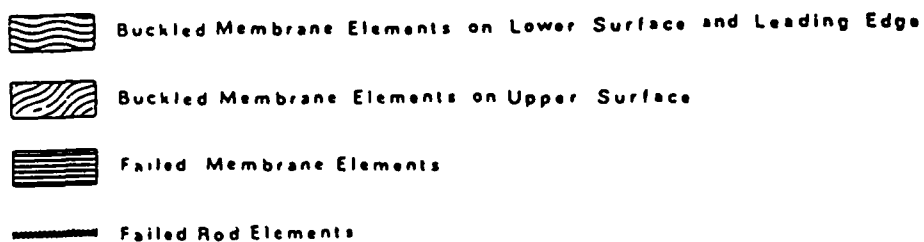
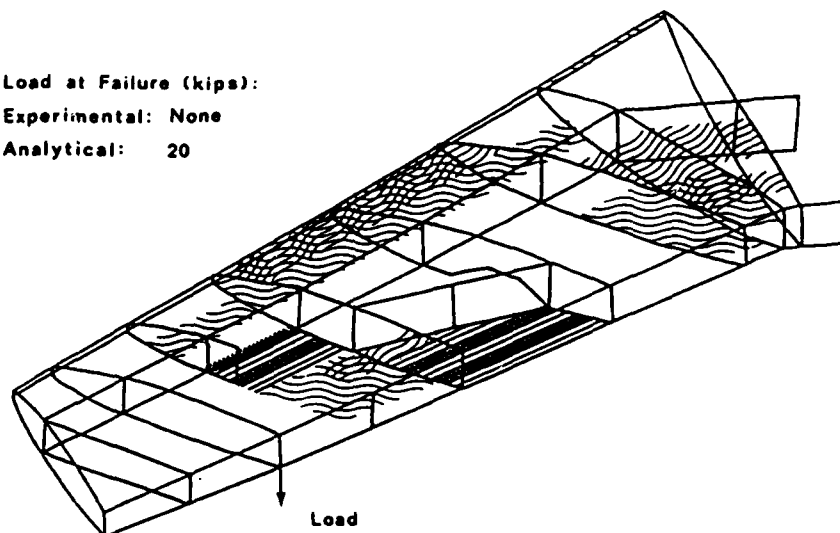
Figure 18. Wing Model Results at Test 2C Failure



Load at Failure (kips):

Experimental: None

Analytical: 20



(b) Model C (without torsional stiffness rods)

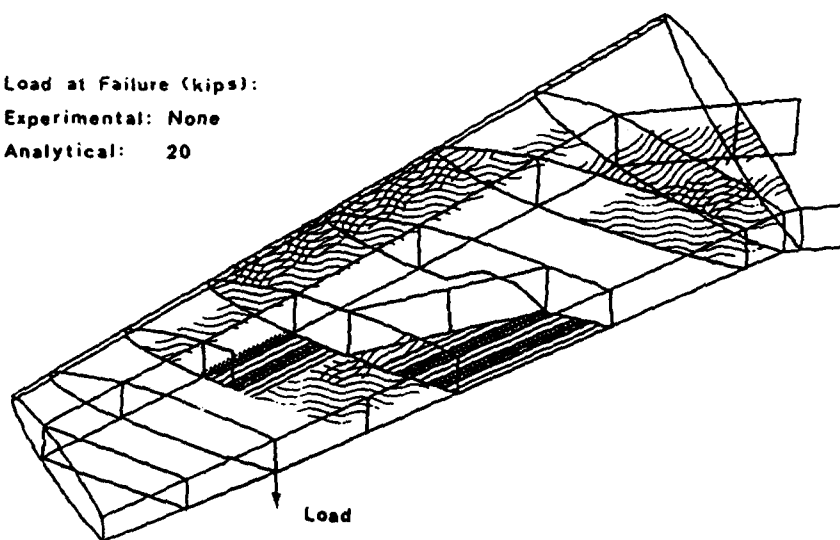
Figure 18. (Continued)







Load at Failure (kips):

Experimental: None

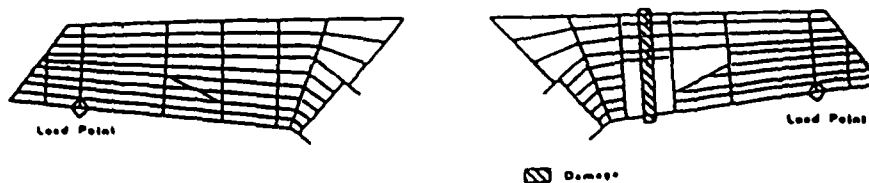
Analytical: 20



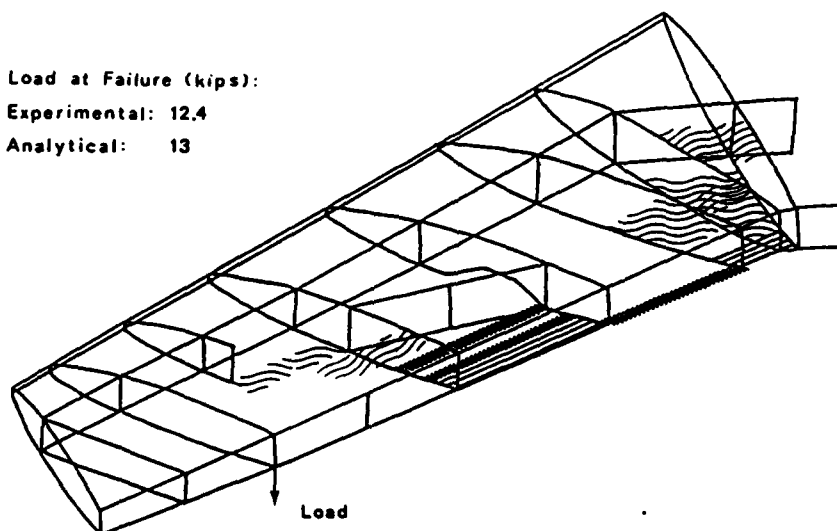
-  Buckled Membrane Elements on Lower Surface and Leading Edge
-  Buckled Membrane Elements on Upper Surface
-  Failed Membrane Elements
-  Failed Rod Elements





(c) Model D (with torsional stiffness rods)

Figure 18. (Continued)



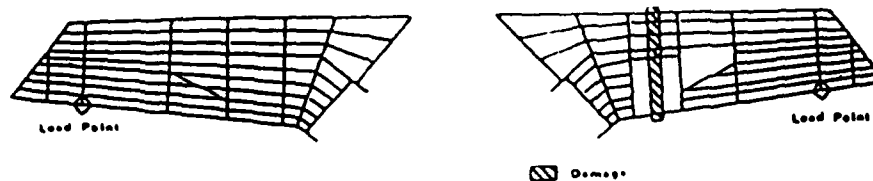
Load at Failure (kips):
 Experimental: 12.4
 Analytical: 13



-  Buckled Shear Panel Elements on Lower Surface and Leading Edge
-  Buckled Shear Panel Elements on Upper Surface
-  Failed Shear Panel Elements
-  Failed Rod Elements

(a) Model A (without torsional stiffness rods)

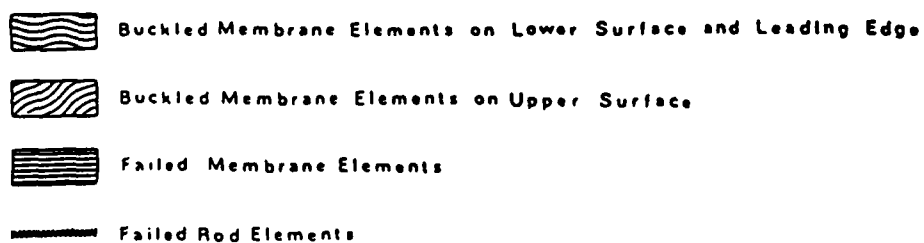
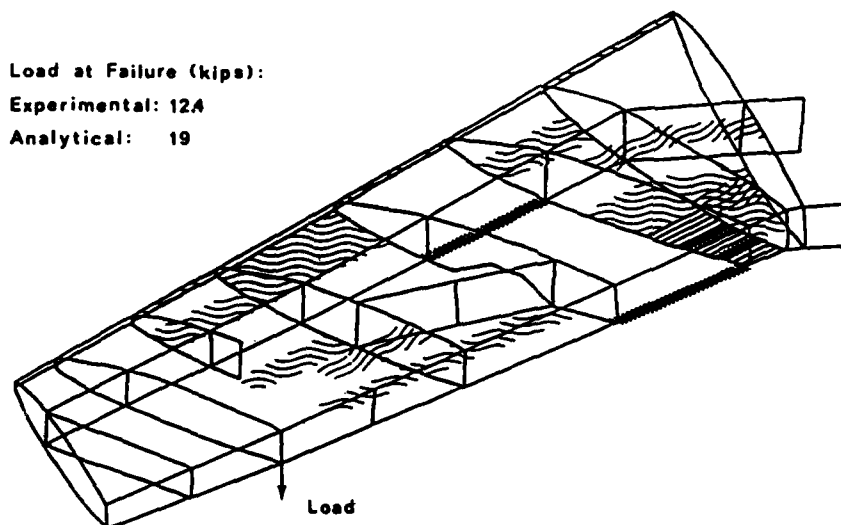
Figure 19. Wing Model Results at Test 3B Failure



Load at Failure (kips):

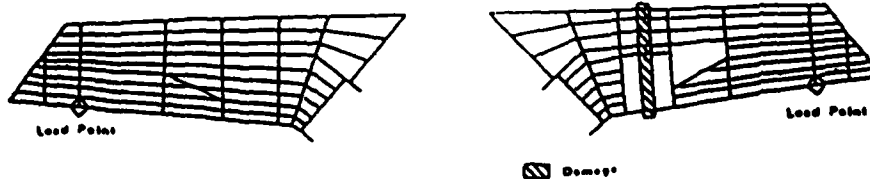
Experimental: 12.4

Analytical: 19

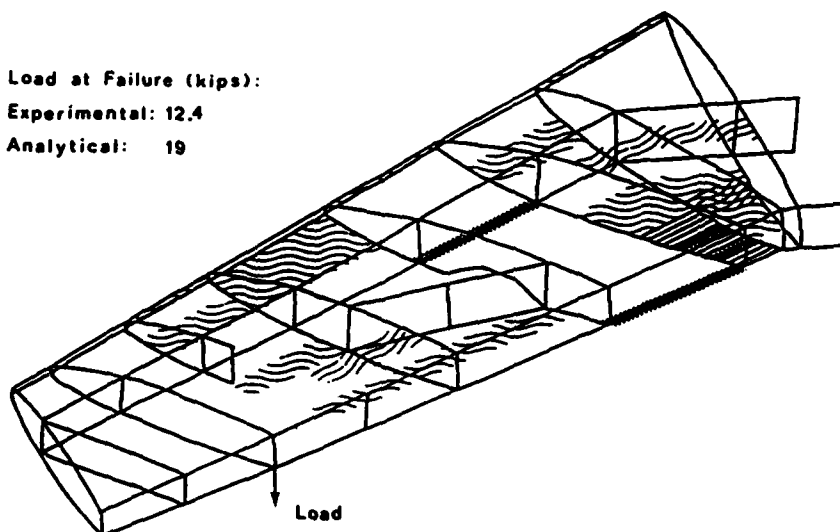






(b) Model C (without torsional stiffness rods)

Figure 19. (Continued)



Load at Failure (kips):
 Experimental: 12.4
 Analytical: 19



-  Buckled Membrane Elements on Lower Surface and Leading Edge
-  Buckled Membrane Elements on Upper Surface
-  Failed Membrane Elements
-  Failed Rod Elements

(c) Model D (with torsional stiffness rods)

Figure 19. (Continued)

Of the two approaches using Model D, the simpler approach depicted more accurately the failure as observed in the laboratory. The simple approach applied to Model A did not produce an accurate failure pattern; however, all tests indicated that Model A was less suitable for predicting the pattern of failure.

Although the Model D results suggested a preference for the simpler technique, caution is advised before reaching a firm conclusion. In all models for all tests, the webs of spars and ribs were represented by shear panels. Consequently, the front spar in Test 1 could not fail at the damage until shear limits were exceeded. The experimental program showed the damaged front spar web in Test 1 failed in bending tension, a failure mode the shear panel could not predict. For cases of initial damage where all or most of a spar cap or rib cap would be removed, the web should probably be modeled by a membrane element. Although the membrane element would be stiffer than the shear panel, it would be directly sensitive to limiting tensile and compressive stresses as well as to shear limits. Such a recommendation applied to this study may have appreciably reduced the predicted failure load for Test 1, and it may have altered the apparent value of simple modeling over the more detailed representation.

8.6 Rotation of Wing Spar Roots

Specific values of displacement were of interest in this study as an additional means of comparing analytical results to laboratory data. To obtain more accurate displacement values from the analytical method, rotations of wing spar roots were measured in the experimental test program and enforced in the analytical models. However, for most applications of

the AFATL method, rotations at structure supports would not be known. Additionally, the precise displacements of the structure probably would be unimportant. The displaced shape of the structure, which might be used to modify loading for each iteration, was available from the analyses using zero support rotations.

The enforced rotations were derived from experimental data. In translating laboratory measurements into single point constraints for NASTRAN, an assumption was made. It was assumed that the center of rotation for each spar was the point midway between the two pins securing the spar in its support structure. In fact, any point between those two pins could have been the center of rotation, and the center could have changed as loading progressed. The assumption almost certainly contributed to the introduction of erroneous stresses into the models during the first set of analyses.

Another likely contributor to those stresses was the manner in which some of the multipoint constraint equations for the models were written. A spar root was modeled by a shear panel and two rod elements, a configuration that gave the desired resistance to bending but provided no lateral restraint. The necessary lateral restraint was provided by multipoint constraints to keep each root section in line with its adjacent spar section.

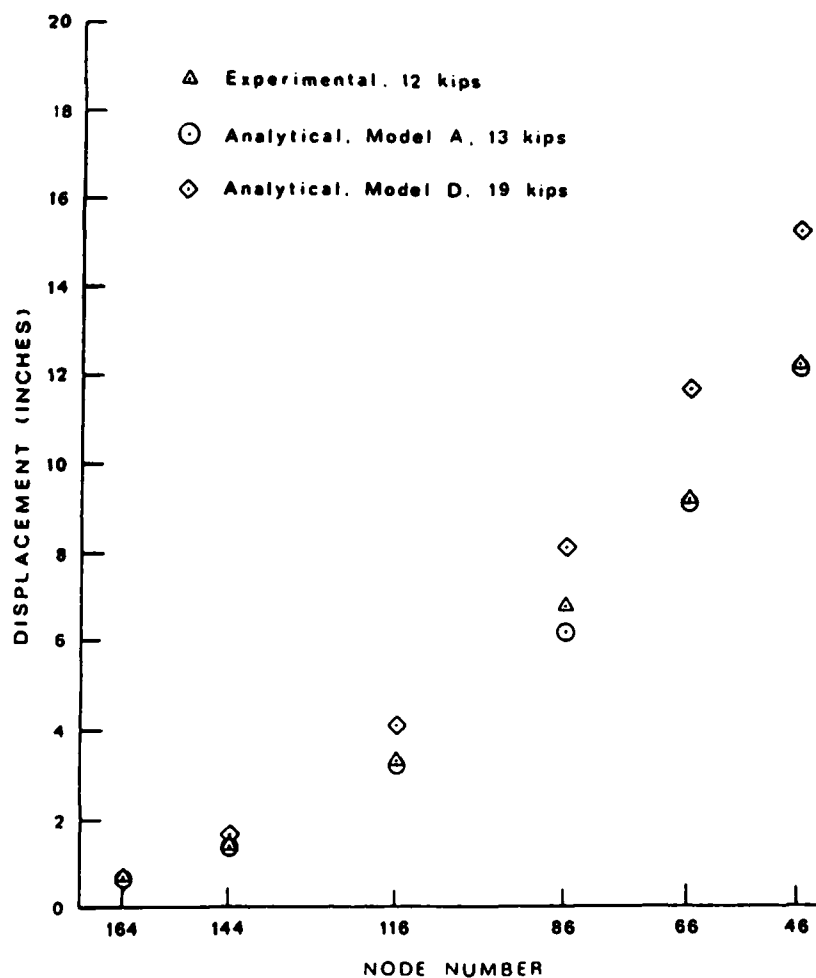
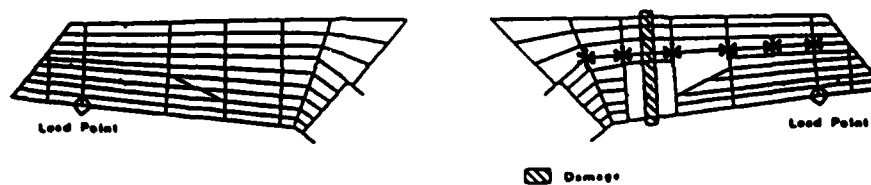
The multipoint constraint equations for the original model were formulated not in a general manner, but with an implied assumption that there was no displacement of the wing spar root nodes. Thus any attempt to enforce the measured rotations violated that assumption. The result was erroneous stresses near the base of the wing.

Of the two sources of error identified, the multipoint constraint equations could be easily corrected. The problem of precisely measuring wing spar root rotations cannot be solved without sophisticated measuring equipment. The benefits gained from precise measurements, however, would not begin to justify the added expense for normal applications of the method.

8.7 Deflections

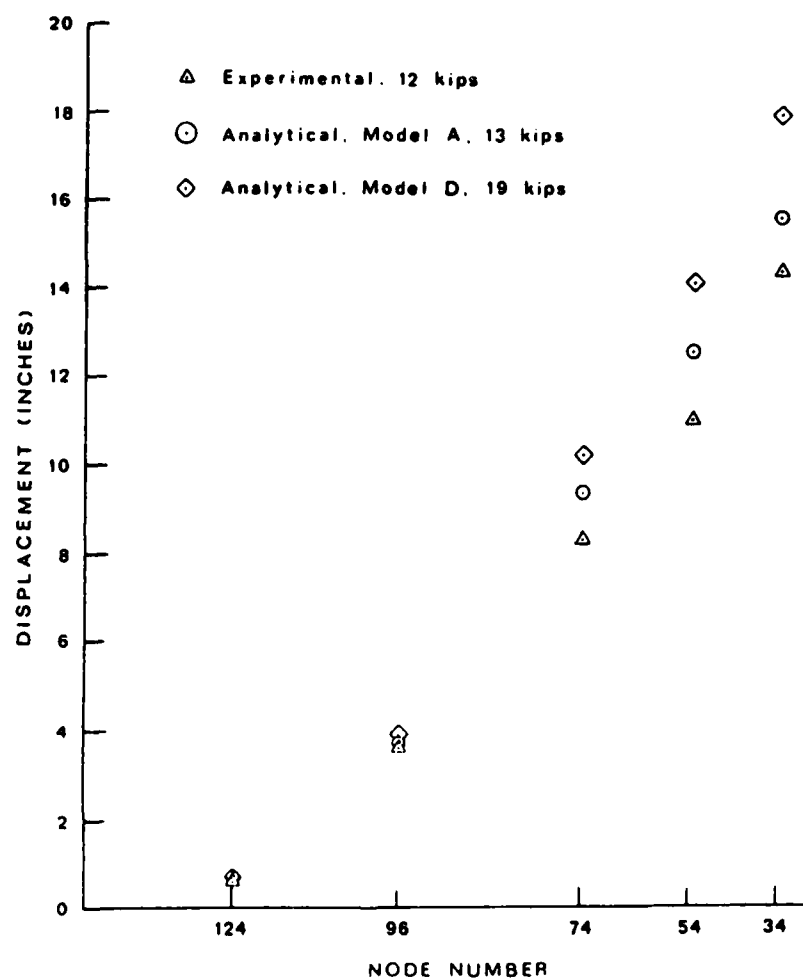
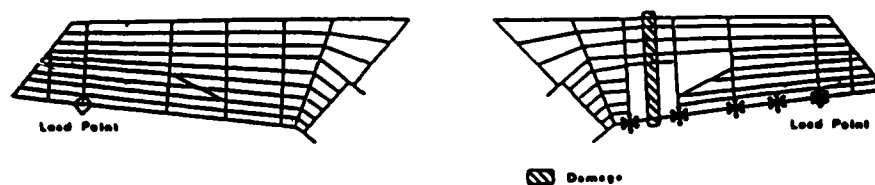
Deflections were measured in the experimental test program and were compared to analytical results. Figures 30 through 32 (Appendix I) present single-point deflection data for Models A, C, and D, and for the test program. Although Model D was selected as the best model because of its ability to predict the failure most realistically, Model A was superior for predicting displacement values. For general deflected shape, however, there was little difference between Models A and D. Figure 20 compares Test 3B profiles of the front and rear spars for Models A and D, and for the actual wing at their respective failure loads. Both models presented essentially the same deflected shape which differed only slightly from measured results.

As mentioned in the previous section, deflections are not envisioned as a critical factor in the routine application of the AFATL method. Even if deflected shape were important, Models A and D both returned approximately the same results. If specific values of displacement were to become the overriding concern in a specialized application, Model A would appear to be the better model. Otherwise, Model D provided reasonable accuracy for deflected shape.



(a) Front Spar Profile

Figure 20. Failure Load Profiles for Test 3B



(b) Rear Spar Profile

Figure 20. (Continued)

CHAPTER IX

SUMMARY AND CONCLUSIONS

The principal goal of this study was to evaluate the suitability of the AFATL method for predicting progressive collapse in complex structures. Suitability was to be investigated by determining supportable limiting stress values, selecting a good combination of finite elements for modeling, and comparing analytical to experimental results.

Limiting stresses used in this study were a direct application of classical theory. Consequently, any skilled analyst could apply the concepts to any structure. A conscious effort was made to found the work in commonly known principals of materials behavior and to avoid the structure-dependency associated with empirical formulations.

All models evaluated used axial rod and shear panel element combinations to represent spars and ribs. Model D used membrane elements to model aircraft skin and rod elements to model skin stiffeners. Additionally, it had torsional rod elements along the spar centerlines. Model A used shear panels and thickened rod elements to represent aircraft skin and skin stiffeners. Model A had no torsional rod elements.

Models A and D both overestimated the residual strength of the damaged structures. For the purposes of this study, those results were conservative. A deficiency observed in the study was the lack of consistency in the degree to which residual strength was overestimated. However, all estimates were within a factor of two of the experimental results.

Model A provided better deflection estimates than Model D. Using shear panel elements for the aircraft skin made Model A more difficult to prepare than the models using membrane elements. As explained in Appendix C, the use of shear panel elements required additional calculations for modifying rod element sizes to represent the membrane capacity of the skin. However, once Model A was developed, it was less expensive to use than models with membrane skin elements. For applications where structure displacements are of primary concern, Model A would provide better results.

Model D described more accurately the actual pattern of failure leading to structural collapse. Using membrane elements for the aircraft skin made Model D a simpler model to prepare as described in Appendix C. The membrane elements also gave a better qualitative representation of skin panel behavior. For applications where the failure pattern is of principal interest, Model D would provide better results.

PROSCAN was developed as a convenience to automate the application of the AFATL method. It proved to be more of a necessity than a convenience in processing the volumes of data generated by many iterative finite element analyses. Additionally, it provided flexibility in the selection of loading sequences and in the application of limiting stresses for elements.

The combination of automation, modeling techniques, and limiting stresses applied to the AFATL method produced a useful estimating tool for predicting progressive collapse in complex structures such as the F-84F aircraft wing. The F-84F wing is a semi-monocoque structure with a heavy two-spar skeletal frame. To further evaluate the versatility of

the method, it should also be tested using other types of structures such as different aircraft designs and components or building structures.

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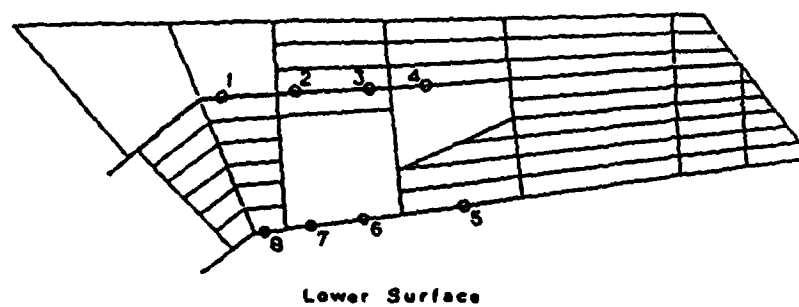
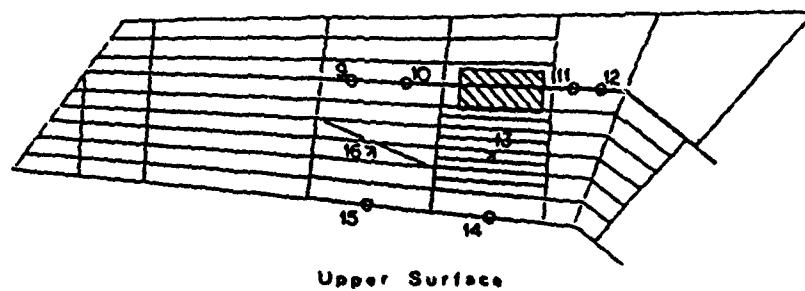
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APPENDIX A

STRAIN GAGE LOCATIONS FOR EXPERIMENTAL PROGRAM

Three wings were tested in the experimental portion of the study. Surface strains were measured using strain gages manufactured by Micro-Measurements of Romulus, Michigan. Two types of gages were used: EA-13-125AD-120 uniaxial gages, and EA-13-250RA-120 three-gage rectangular rosettes. Figure 21 details strain gage locations for all three tests.

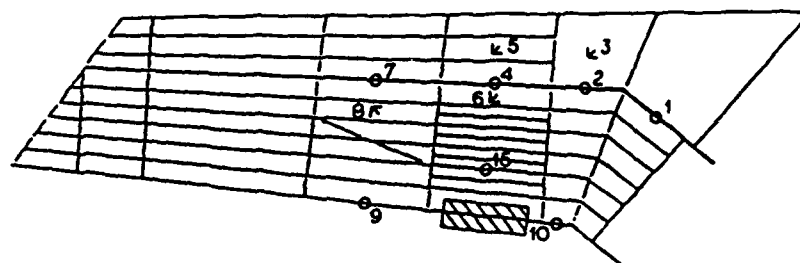


⊖ Uniaxial

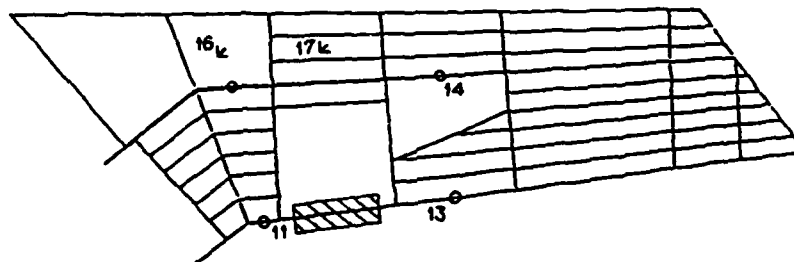
⌄ Rectangular Rosette

(a) Test 1

Figure 21. Strain Gage Locations



Upper Surface



Lower Surface

⊖ Uniaxial

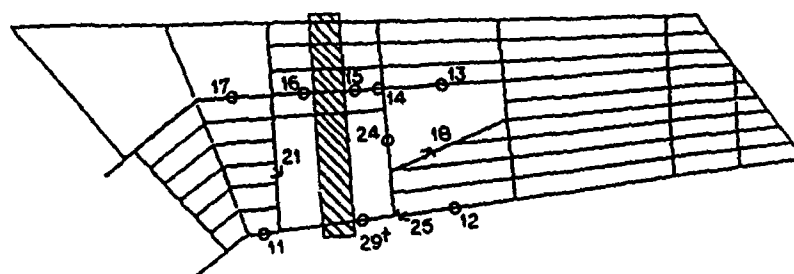
⌞ Rectangular Rosette

(b) Test 2

Figure 21. (Continued)



Upper Surface



Lower Surface

⊖ Uniaxial

⌞ Rectangular Rosette

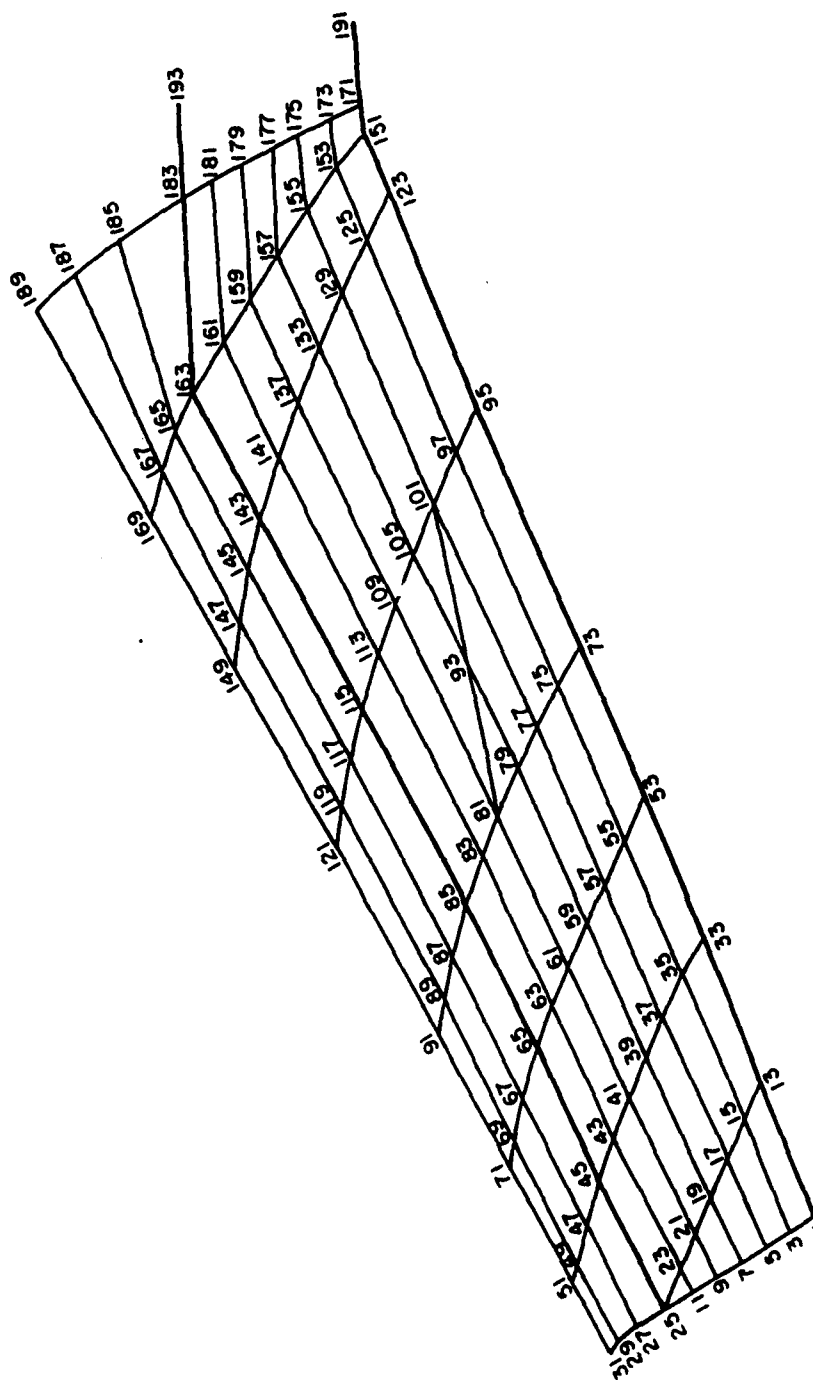
† Test 3B only

(c) Test 3

Figure 21. (Continued)

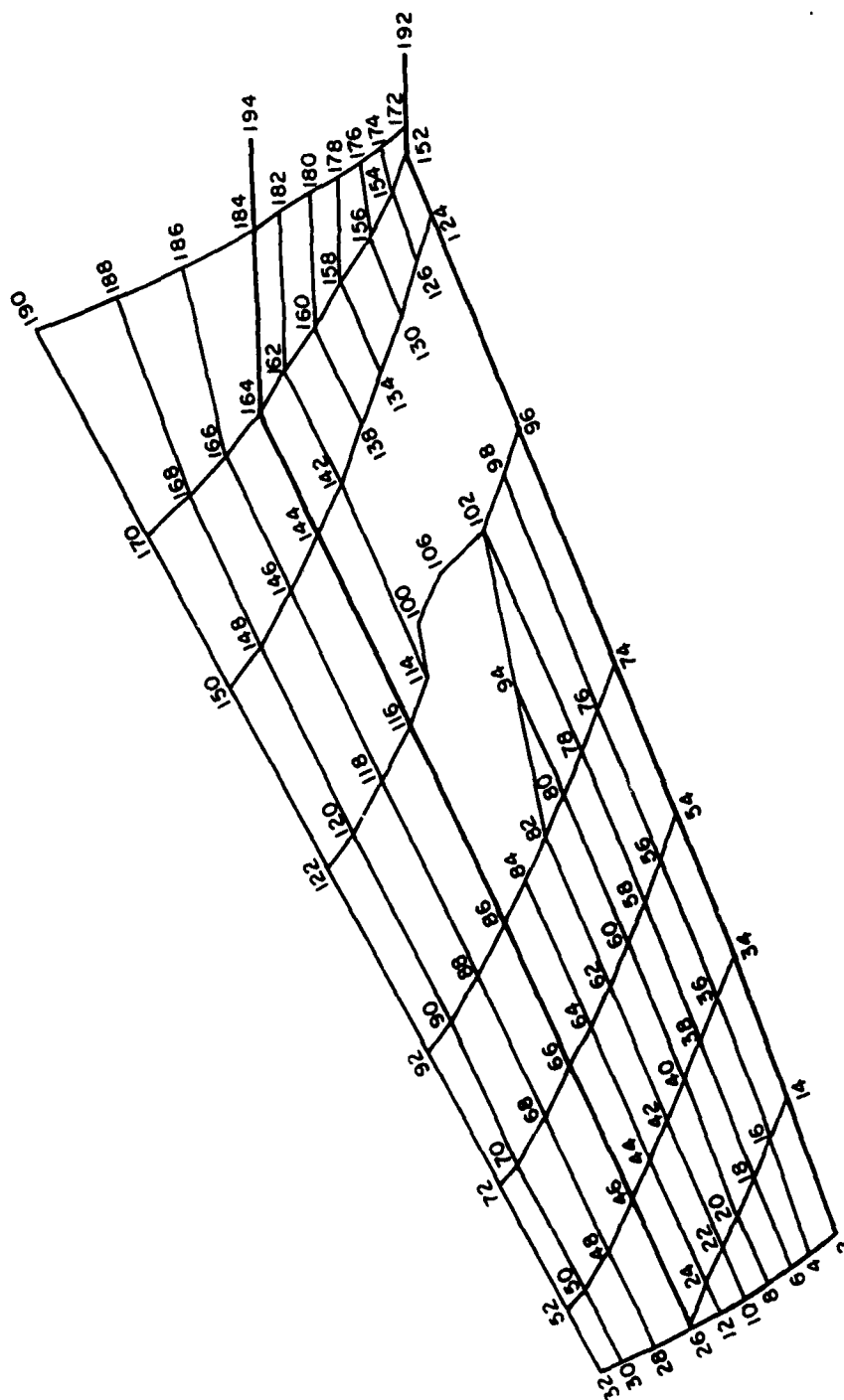
APPENDIX B

FINITE ELEMENT MODEL NUMBERING DETAILS



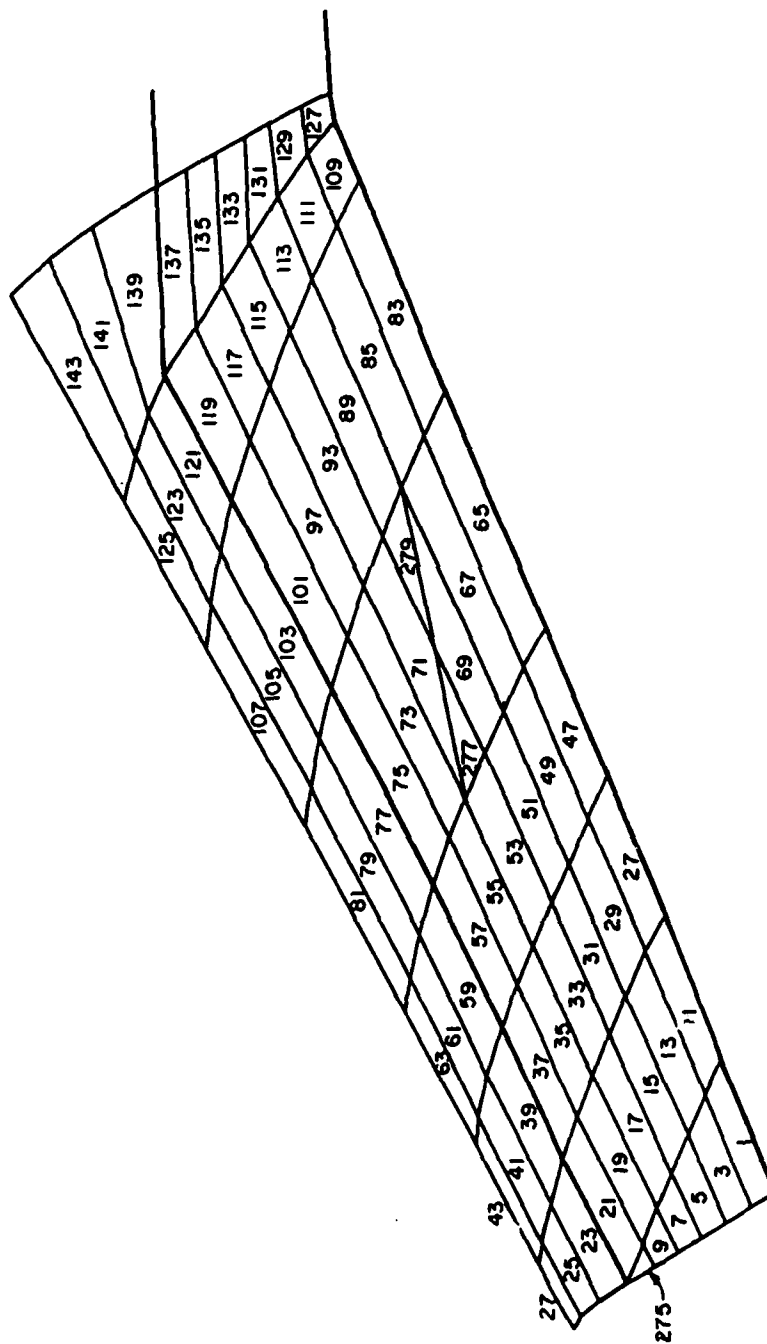
a. Node Numbers, Upper Surface

Figure 22. Finite Element Model Numbering Details



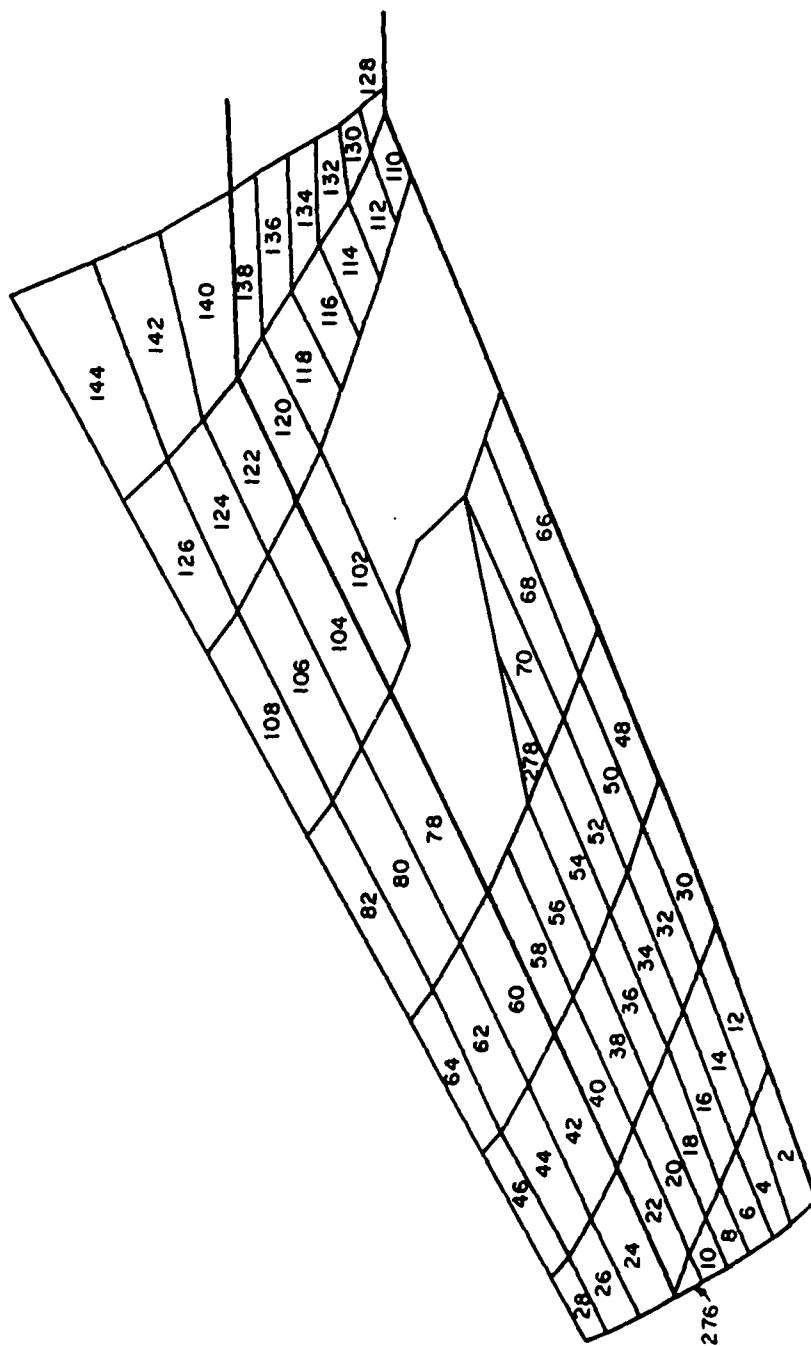
b. Node Numbers, Lower Surface

Figure 22. (Continued)



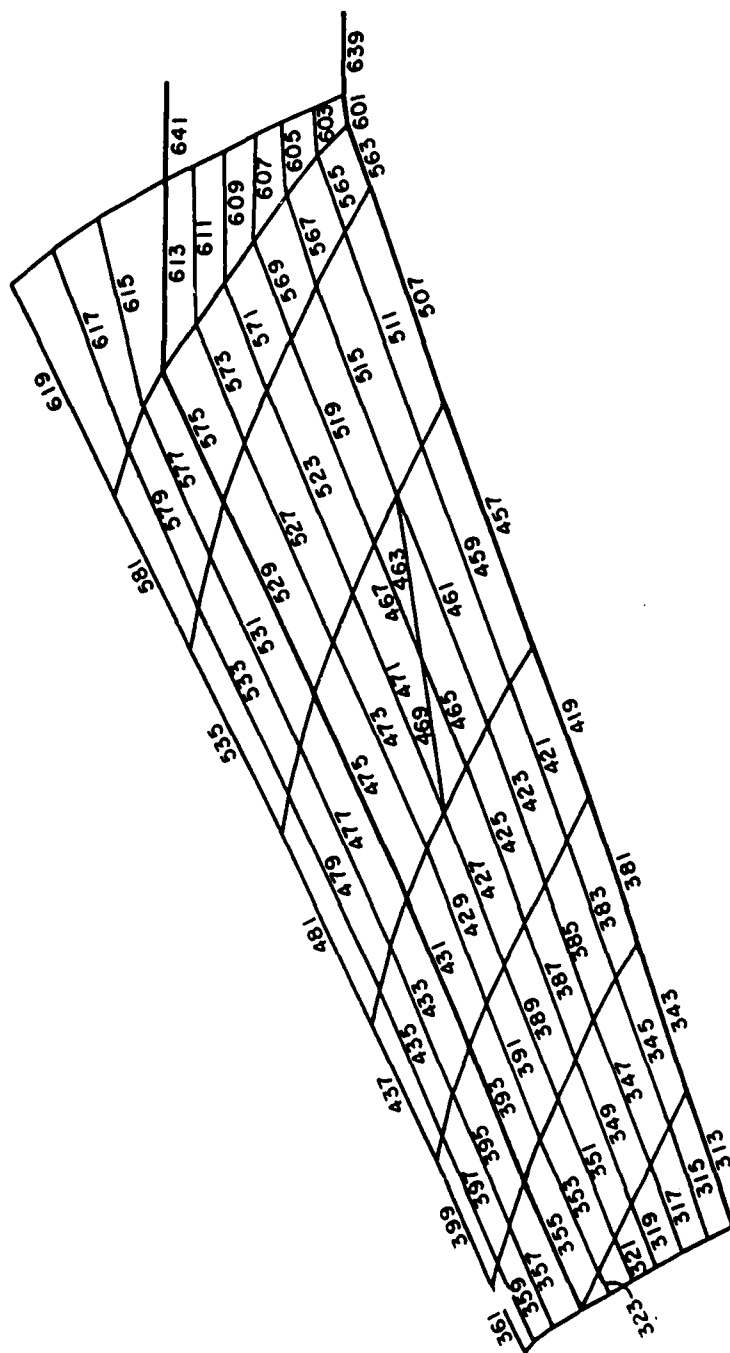
c. Skin Panels, Upper Surface

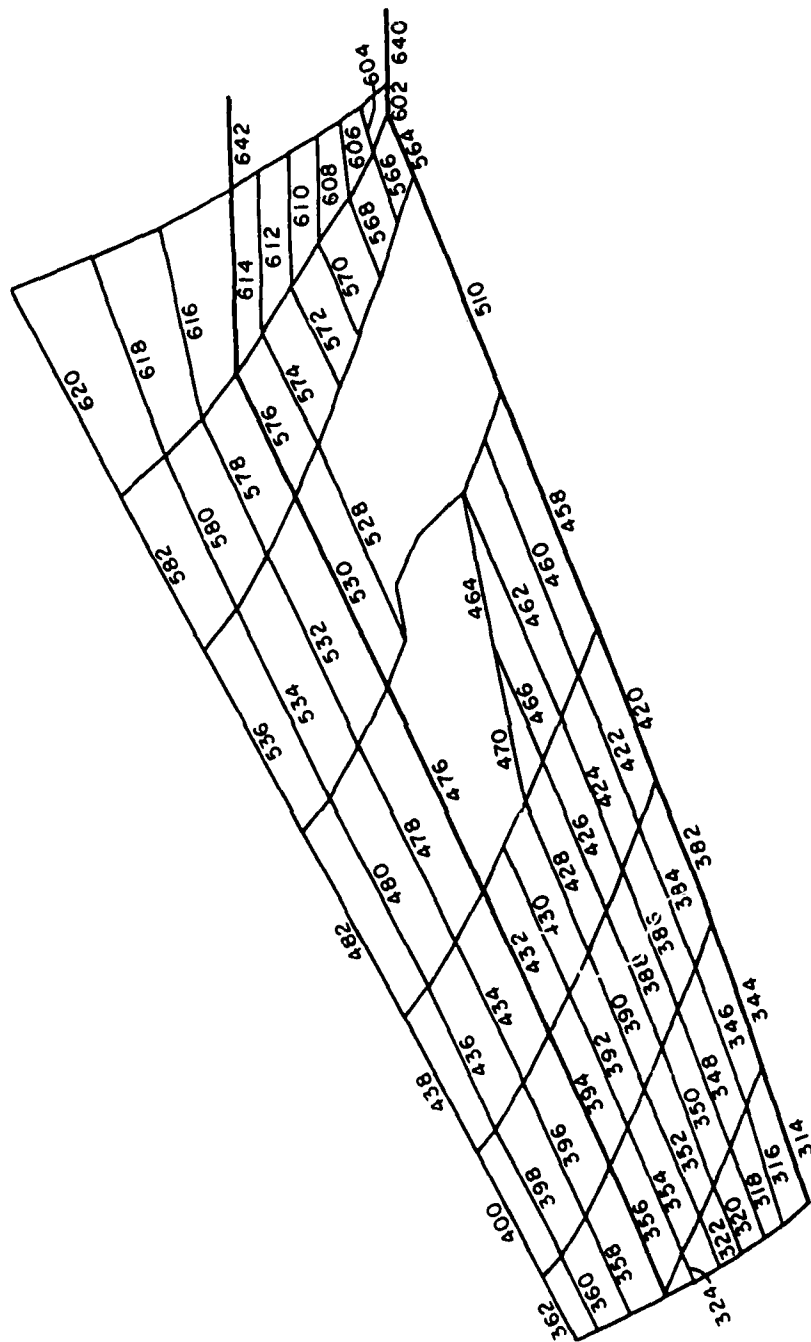
Figure 22. (Continued)



d. Skin Panels, Lower Surface

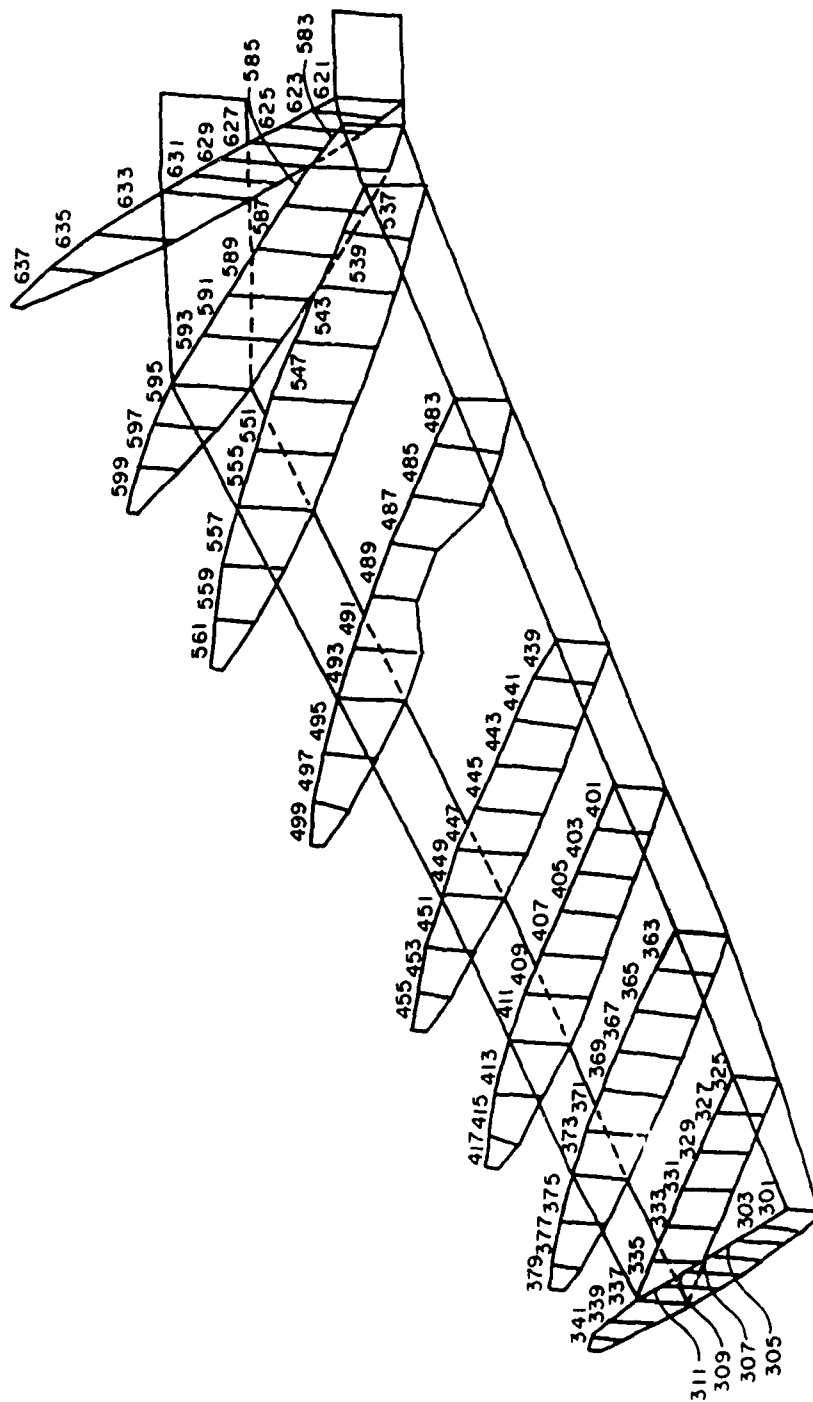
Figure 22. (Continued)





f. Spar cap and Stiffener Rods, Lower Surface

Figure 22. (Continued)



9. Rib Cap Rods, Upper Surface

Figure 22. (Continued)

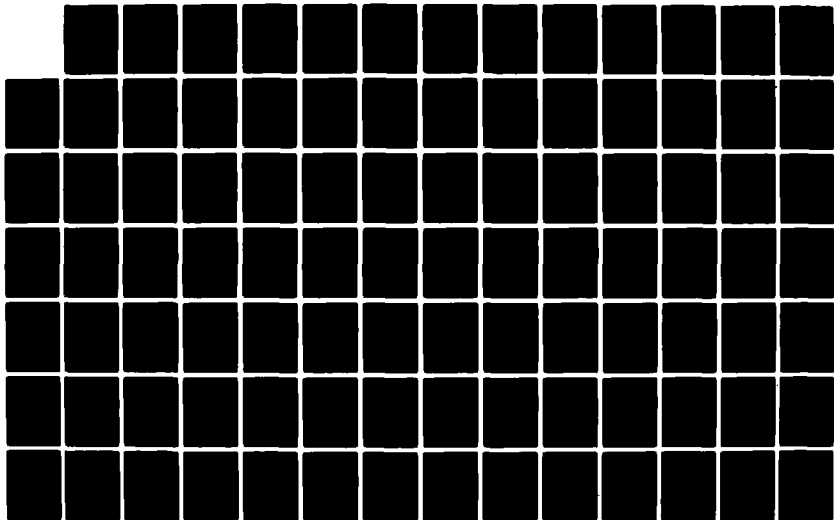
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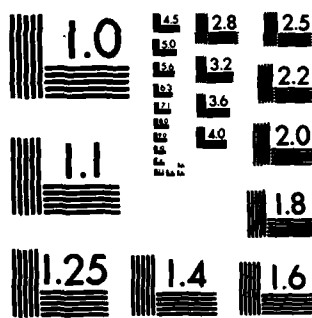
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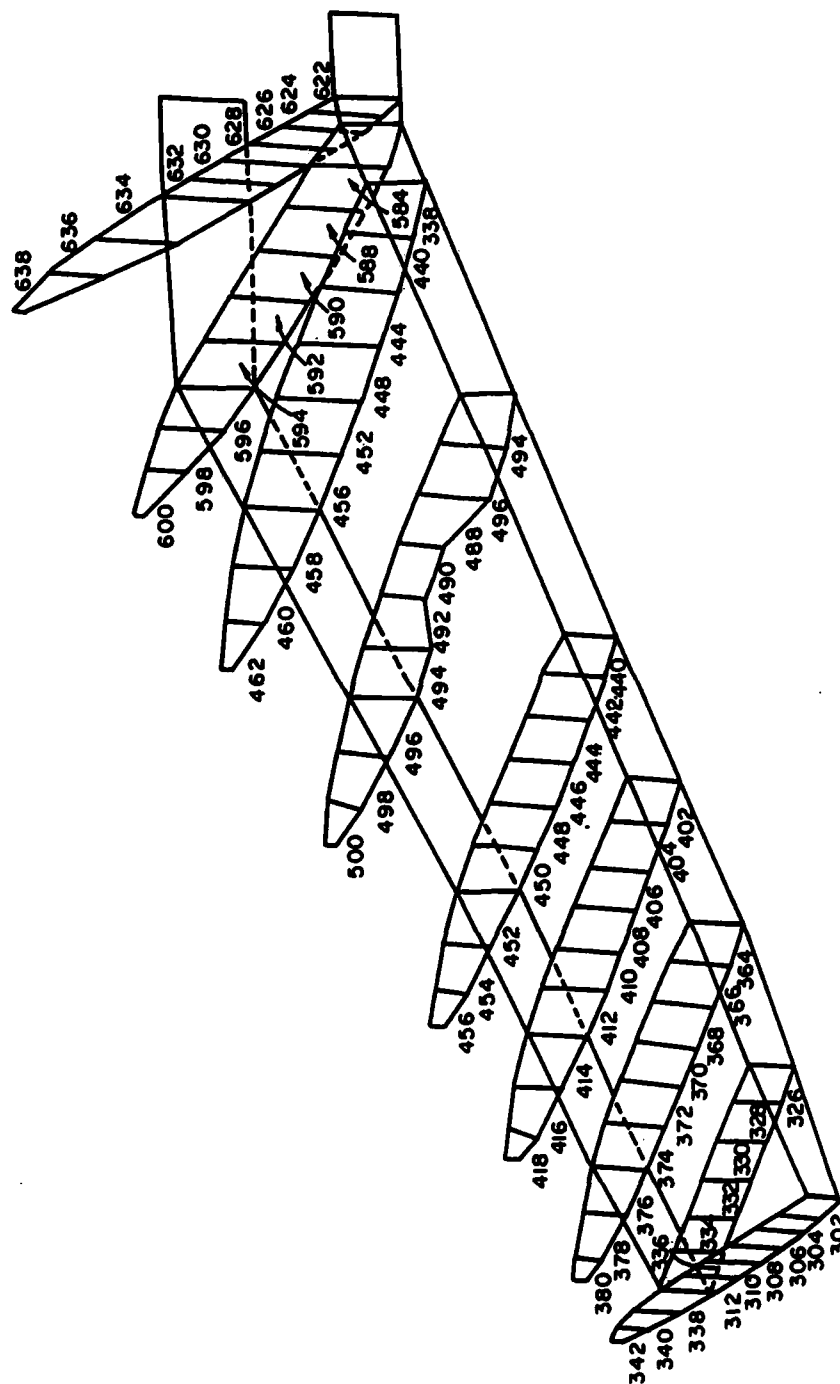
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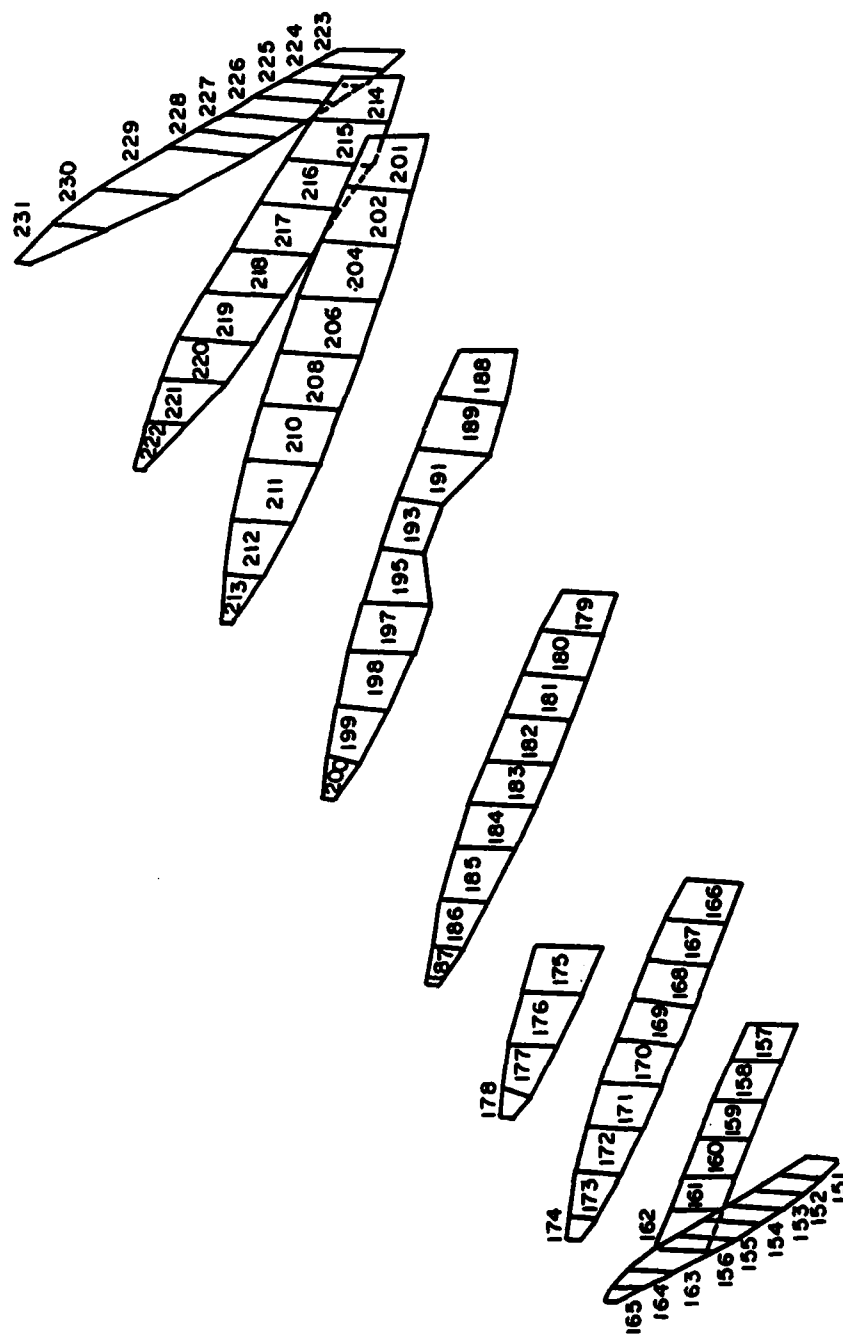


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NATIONAL BUREAU OF STANDARDS-1963-A



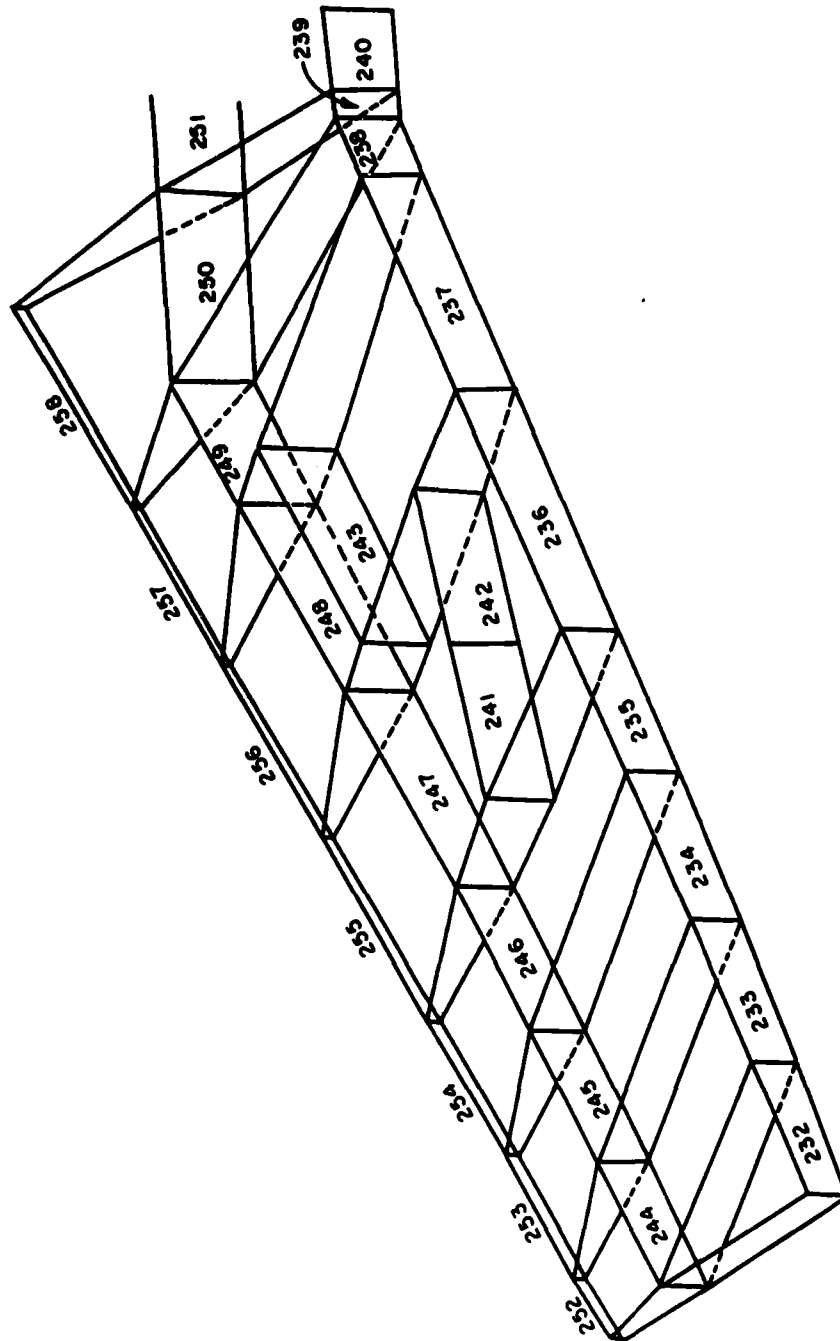
h. Rib Cap Rods, Lower Surface

Figure 22. (Continued)



i. Rib Web Elements

Figure 22. (Continued)



j. Spar Web and Leading Edge Elements

APPENDIX C

SIZING OF ROD ELEMENTS

The rationale for sizing rod elements representing spar caps or rib caps was presented by Jordan (32, 33). All symbols in the following summary of his presentation refer to Figures 23 and 24 which were extracted from Reference (32).

Property relationships were first determined for a structural member's cross section. The distance from the top surface of the section to the centroidal axis was identified as h_t . Similarly, the distance from the centroidal axis to the bottom surface of the section was labeled h_b . Maximum bending stresses at top and bottom surfaces, respectively, were

$$\sigma_T = \frac{Mh_t}{I} \quad (A.1a)$$

and

$$\sigma_B = \frac{Mh_b}{I} \quad (A.1b)$$

where M was the applied bending moment, and I was the section moment of inertia about the centroidal axis.

In the model, the rod elements were assumed to be point areas and were positioned at the top and bottom surfaces of a cross section. The rod areas were sized to maintain the location of the centroidal axis and the value of I for the actual section. Such a relationship yielded

$$A_T h_t = A_B h_b \quad (A.2)$$

with A_T and A_B being the areas of the top and bottom rods. The bending moment, M , in the model then became

$$M = \sigma_T A_T h_t + \sigma_B A_B h_b \quad (A.3)$$

Next, using actual section properties and desired model properties, the

relationships of Equations (A.1), (A.2), and (A.3) were combined to produce

$$A_T = \frac{I}{h_t(h_t + h_b)} \quad (A.4a)$$

and

$$A_B = \frac{I}{h_b(h_t + h_b)} \quad (A.4b)$$

which were the rod element areas.

Skin stiffeners also were represented by rod elements. Initially, each of those rods had the same cross sectional area and position in the structure as the stiffener it represented. However, Models A and B used shear panel elements to represent aircraft skin, so the membrane capacity of the skin panels was lost because shear panel elements do not represent membrane behavior. The membrane capacity of the skin was restored to models A and B by adding the cross sectional area of each skin panel to its bordering rod elements. This is illustrated for a typical cross section in Figure 24. Such modification of stiffener rod element areas was not necessary for Models C and D because they used membrane elements to represent the aircraft skin.

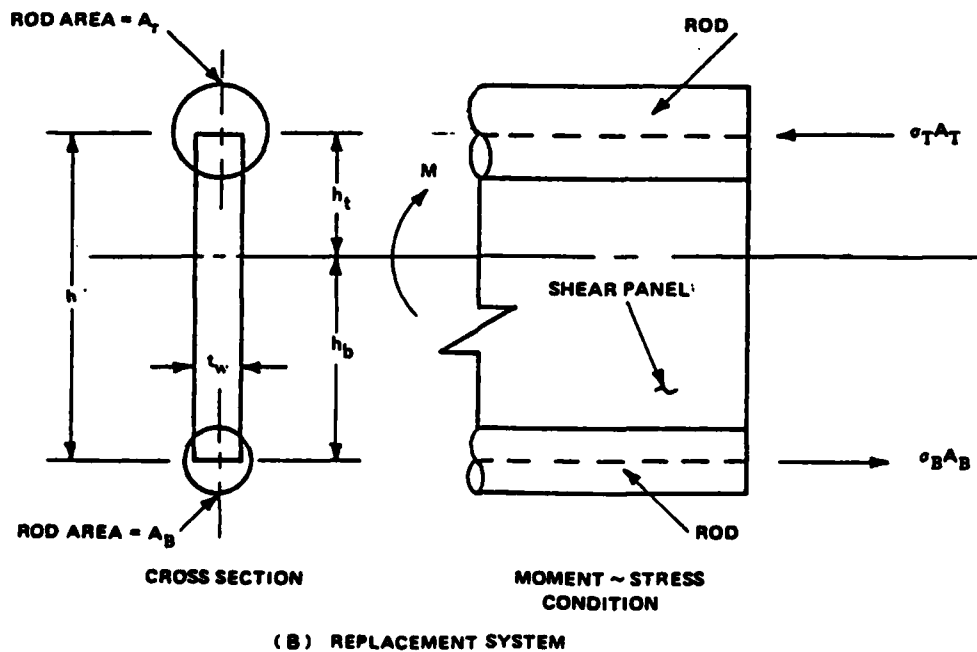
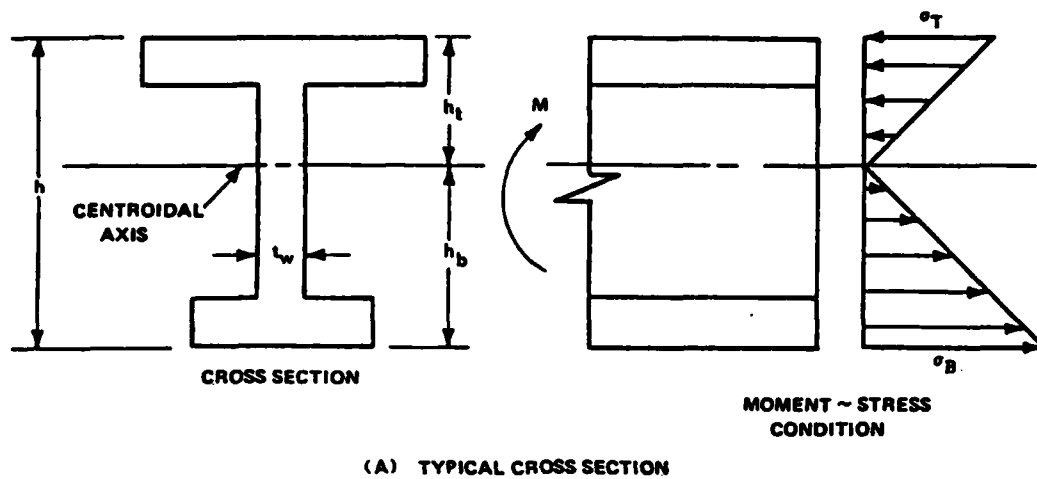


Figure 23. Basic Sizing of Rod Elements

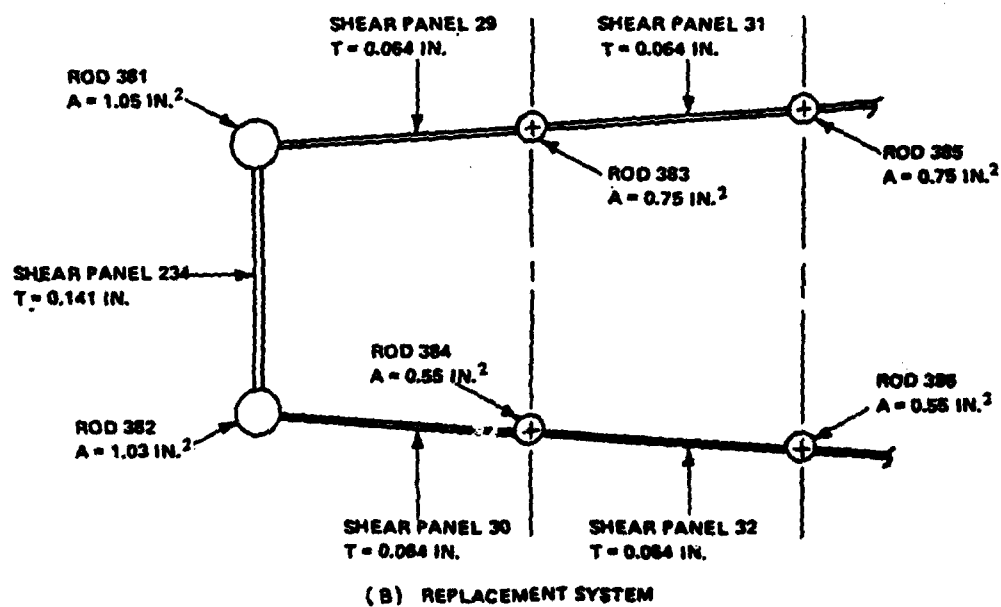
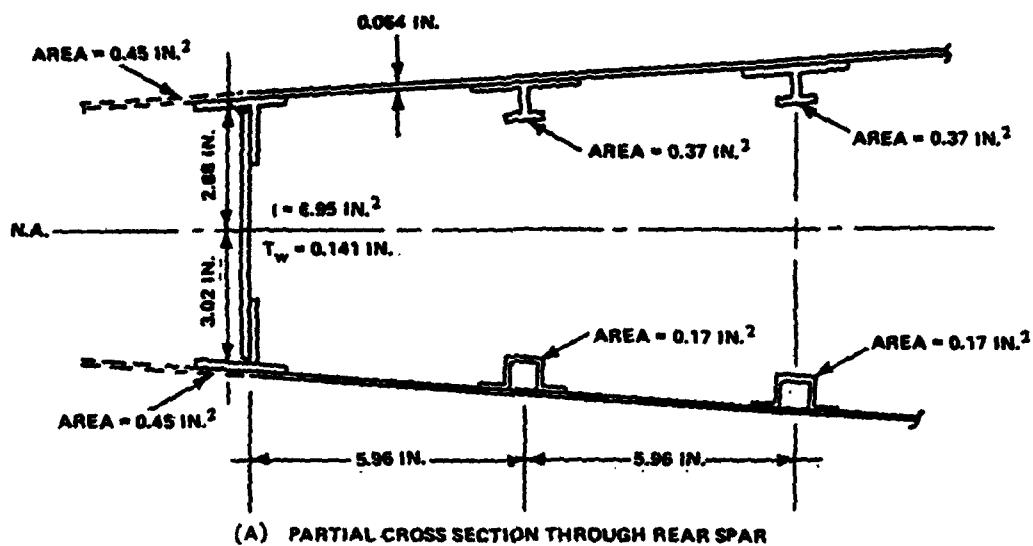


Figure 24. Rod Element Sizing With Shear Panel Skin

APPENDIX D

FINITE ELEMENT MODEL A LISTING

CH88	1115	1	29	30	1116	43	31	32	P888	121	11	1.04
CH88	1117	1	33	34	1118	1	35	36	P888	122	11	1.05
CH88	1119	1	37	38	1120	1	39	40	P888	123	11	1.06
CH88	1121	1	41	42	1122	1	43	44	P888	124	11	1.07
CH88	1123	1	45	46	1124	1	47	48	P888	125	11	1.33
CH88	1125	1	49	50	1126	67	51	52	P888	126	11	1.37
CH88	1127	1	53	54	1128	1	55	56	P888	127	11	1.38
CH88	1129	1	57	58	1130	1	59	60	P888	128	11	1.40
CH88	1131	1	61	62	1132	73	71	72	P888	129	11	1.41
CH88	1133	1	73	74	1134	1	75	76	P888	130	11	1.48
CH88	1135	1	77	78	1136	1	79	80	P888	131	11	1.50
CH88	1137	1	81	82	1138	1	83	84	P888	132	11	1.68
CH88	1139	1	85	86	1140	1	87	88	P888	133	11	1.68
CH88	1141	1	89	90	1142	9	91	92	P888	134	11	1.73
CH88	1143	1	93	94	1144	1	95	96	P888	135	11	1.75
CH88	1145	1	97	98					P888	136	11	1.78
CH88	1147	1	101	102					P888	137	11	1.82
CH88	1149	1	105	106					P888	138	11	1.84
CH88	1151	1	109	110					P888	139	11	1.94
CH88	1153	1	113	114	1154	1	115	116	P888	140	11	1.96
CH88	1155	1	117	118	1156	1	119	120	P888	141	12	0.29
CH88	1157	97	121	122	1158	1	123	124	P888	142	12	0.29
CH88	1159	1	125	126					P888	143	12	0.52
CH88	1161	1	129	130					P888	144	12	0.70
CH88	1163	1	133	134					P888	145	12	0.77
CH88	1165	1	137	138					P888	146	12	0.82
CH88	1167	1	141	142	1170	1	147	148	P888	147	12	0.85
CH88	1169	1	145	146	1172	1	149	150	P888	148	12	1.07
CH88	1171	95	149	150	1174	1	151	152	P888	149	12	1.09
CH88	1173	1	153	154	1176	1	155	156	P888	150	12	1.20
CH88	1175	1	157	158					P888	151	12	1.76
CH88	1177	1	161	162					P888	152	12	1.76
CH88	1179	1	165	166	1180	1	167	168	P888	153	12	1.79
CH88	1181	99	169	170	1182	1	171	172	P888	154	12	0.26
CH88	1183	1	173	174	1184	1	175	176	P888	155	12	0.42
CH88	1185	1	177	178	1186	1	179	180	P888	156	13	0.15
CH88	1187	1	181	182	1188	1	183	184	P888	157	13	0.43
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CH88	1195	1	197	198					P888	161	13	1.18
CH88	1197	1	201	202					P888	162	13	1.49
CH88	1199	1	205	206					P888	163	13	1.49
CH88	1201	1	209	210					P888	164	13	1.49
CH88	1203	1	213	214					P888	165	13	1.57
CH88	1205	1	217	218					P888	166	13	1.58
CH88	1207	1	221	222					P888	167	13	1.58
CH88	1209	1	225	226					P888	168	13	2.17
CH88	1211	1	229	230					P888	169	13	2.17
CH88	1213	1	233	234					P888	170	13	2.57
CH88	1215	1	237	238					P888	171	13	2.58
CH88	1217	1	241	242					P888	172	13	2.58
CH88	1219	1	245	246					P888	173	13	2.58
CH88	1221	1	249	250					P888	174	13	2.58
CH88	1223	1	253	254					P888	175	13	3.27
CH88	1225	1	257	258					P888	176	13	3.27
CH88	1227	1	261	262					P888	177	13	3.27
CH88	1229	1	265	266					P888	178	13	3.27
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CH88	1233	1	273	274					P888	180	13	3.27
CH88	1235	1	277	278					P888	181	13	3.27
CH88	1237	1	281	282					P888	182	13	3.27
CH88	1239	1	285	286					P888	183	13	3.27
CH88	1241	1	289	290					P888	184	13	3.27
CH88	1243	1	293	294					P888	185	13	3.27
CH88	1245	1	297	298					P888	186	13	3.27
CH88	1247	1	301	302					P888	187	13	3.27
CH88	1249	1	305	306					P888	188	13	3.27
CH88	1251	1	309	310					P888	189	13	3.27
CH88	1253	1	313	314					P888	190	13	3.27
CH88	1255	1	317	318					P888	191	13	3.27
CH88	1257	1	321	322					P888	192	13	3.27
CH88	1259	1	325	326					P888	193	13	3.27
CH88	1261	1	329	330					P888	194	13	3.27
CH88	1263	1	333	334					P888	195	13	3.27
CH88	1265	1	337	338					P888	196	13	3.27
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CH88	1271	1	349	350					P888	199	13	3.27
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CH88	1279	1	365	366					P888	203	13	3.27
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CH88	1283	1	373	374					P888	205	13	3.27
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CH88	1365	1	537	538					P888	246	13	3.27
CH88	1367	1	541	542					P888	247	13	3.27
CH88	1369	1	545	546					P888	248	13	3.27
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PROPERTY CARDS FOR NON ELEMENTS ADJACENT TO DAMAGE

[illegible]

APPENDIX E

FINITE ELEMENT MODEL C LISTING

MAX DATA DECK FOR			GRID POINTS - TOP OF WING		
F-4F WING PROJECT - UNARMED WING					
WING STIFFNESS - STIFF CASE					
SPARE AND WING: CORD AND CORD					
STIFFENERS: CORD					
SKIN PANELS: CORD					
NO TORSIONAL STIFFNESS IN SPARS					
THE WING ALUMINUM IS 7075-T6 EXCEPT 2024-T3 IN A FEW SKIN PANELS					
THE WING SPAR MONTS ARE 8 CH-MD-V AIRCRAFT STEEL					
			GRID POINTS - TOP OF WING		
0010	1	332.9	-20.4	-2.8	
0010	3	229.1	-24.23	-2.9	
0010	5	228.1	-20.27	-3.2	
0010	7	231.1	-15.2	-3.5	
0010	9	217.1	-10.13	-3.88	
0010	11	212.1	-5.07	-4.25	
0010	13	208.1	-2.8	-4.6	
0010	15	200.1	-27.06	-3.08	
0010	17	200.1	-21.67	-3.3	
0010	19	200.1	-16.25	-3.55	
0010	21	200.1	-10.83	-3.85	
0010	23	200.1	-5.47	-4.15	
0010	25	200.1	-0.4	-4.45	
0010	27	200.1	5.4	-4.75	
0010	29	199.9	11.7	-5.05	
0010	31	196.8	18.6	-5.35	
0010	33	184.1	-24.7	-2.85	
0010	35	184.1	-28.92	-3.25	
0010	37	184.1	-22.13	-3.65	
0010	39	184.1	-15.2	-4.05	
0010	41	184.1	-8.25	-4.45	
0010	43	184.1	-1.28	-4.85	
0010	45	184.1	5.78	-5.25	
0010	47	184.1	12.1	-5.65	
0010	49	184.1	18.6	-6.05	
0010	51	184.1	25.0	-6.45	
0010	53	184.1	31.4	-6.85	
0010	55	184.1	37.8	-7.25	
0010	57	184.1	44.2	-7.65	
0010	59	184.1	50.6	-8.05	
0010	61	184.1	57.0	-8.45	
0010	63	184.1	63.4	-8.85	
0010	65	184.1	69.8	-9.25	
0010	67	184.1	76.2	-9.65	
0010	69	184.1	82.6	-10.05	
0010	71	184.1	89.0	-10.45	
0010	73	184.1	95.4	-10.85	
0010	75	184.1	101.8	-11.25	
0010	77	184.1	108.2	-11.65	
0010	79	184.1	114.6	-12.05	
0010	81	184.1	121.0	-12.45	
0010	83	184.1	127.4	-12.85	
0010	85	184.1	133.8	-13.25	
0010	87	184.1	140.2	-13.65	
0010	89	184.1	146.6	-14.05	
0010	91	184.1	153.0	-14.45	
0010	93	184.1	159.4	-14.85	
0010	95	184.1	165.8	-15.25	
0010	97	184.1	172.2	-15.65	
0010	99	184.1	178.6	-16.05	
0010	101	184.1	185.0	-16.45	
0010	103	184.1	191.4	-16.85	
0010	105	184.1	197.8	-17.25	
0010	107	184.1	204.2	-17.65	
0010	109	184.1	210.6	-18.05	
0010	111	184.1	217.0	-18.45	
0010	113	184.1	223.4	-18.85	
0010	115	184.1	229.8	-19.25	
0010	117	184.1	236.2	-19.65	
0010	119	184.1	242.6	-20.05	
0010	121	184.1	249.0	-20.45	
0010	123	184.1	255.4	-20.85	
0010	125	184.1	261.8	-21.25	
			GRID POINTS - BOTTOM OF WING		
0010	1	332.9	-20.4	2.8	
0010	3	229.1	-24.23	2.9	
0010	5	228.1	-20.27	3.2	
0010	7	231.1	-15.2	3.5	
0010	9	217.1	-10.13	3.88	
0010	11	212.1	-5.07	4.25	
0010	13	208.1	-2.8	4.6	
0010	15	200.1	-27.06	3.08	
0010	17	200.1	-21.67	3.3	
0010	19	200.1	-16.25	3.55	
0010	21	200.1	-10.83	3.85	
0010	23	200.1	-5.47	4.15	
0010	25	200.1	-0.4	4.45	
0010	27	200.1	5.4	4.75	
0010	29	199.9	11.7	5.05	
0010	31	196.8	18.6	5.35	
0010	33	184.1	-24.7	2.85	
0010	35	184.1	-28.92	3.25	
0010	37	184.1	-22.13	3.65	
0010	39	184.1	-15.2	4.05	
0010	41	184.1	-8.25	4.45	
0010	43	184.1	-1.28	4.85	
0010	45	184.1	5.78	5.25	
0010	47	184.1	12.1	5.65	
0010	49	184.1	18.6	6.05	
0010	51	184.1	25.0	6.45	
0010	53	184.1	31.4	6.85	
0010	55	184.1	37.8	7.25	
0010	57	184.1	44.2	7.65	
0010	59	184.1	50.6	8.05	
0010	61	184.1	57.0	8.45	
0010	63	184.1	63.4	8.85	
0010	65	184.1	69.8	9.25	
0010	67	184.1	76.2	9.65	
0010	69	184.1	82.6	10.05	
0010	71	184.1	89.0	10.45	
0010	73	184.1	95.4	10.85	
0010	75	184.1	101.8	11.25	
0010	77	184.1	108.2	11.65	
0010	79	184.1	114.6	12.05	
0010	81	184.1	121.0	12.45	
0010	83	184.1	127.4	12.85	
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0010	89	184.1	146.6	14.05	
0010	91	184.1	153.0	14.45	
0010	93	184.1	159.4	14.85	
0010	95	184.1	165.8	15.25	
0010	97	184.1	172.2	15.65	
0010	99	184.1	178.6	16.05	
0010	101	184.1	185.0	16.45	
0010	103	184.1	191.4	16.85	
0010	105	184.1	197.8	17.25	
0010	107	184.1	204.2	17.65	
0010	109	184.1	210.6	18.05	
0010	111	184.1	217.0	18.45	
0010	113	184.1	223.4	18.85	
0010	115	184.1	229.8	19.25	
0010	117	184.1	236.2	19.65	
0010	119	184.1	242.6	20.05	
0010	121	184.1	249.0	20.45	
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0010	125	184.1	261.8	21.25	

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CR00	348	300	38	18	349	201	39	19	119
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CR00	350	300	42	22	351	201	43	23	121
CR00	351	300	43	23	352	201	44	24	122
CR00	352	300	44	24	353	201	45	25	123
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CR00	357	300	49	29	358	201	50	30	128
CR00	358	300	50	30	359	201	51	31	129
CR00	359	300	51	31	360	201	52	32	130
CR00	360	300	52	32	361	201	53	33	131
CR00	361	300	53	33	362	201	54	34	132
CR00	362	300	54	34	363	201	55	35	133
CR00	363	300	55	35	364	201	56	36	134
CR00	364	300	56	36	365	201	57	37	135
CR00	365	300	57	37	366	201	58	38	136
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CR00	367	300	59	39	368	201	60	40	138
CR00	368	300	60	40	369	201	61	41	139
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CR00	371	300	63	43	372	201	64	44	142
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CR00	373	300	65	45	374	201	66	46	144
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CR00	375	300	67	47	376	201	68	48	146
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CR00	382	300	74	54	383	201	75	55	153
CR00	383	300	75	55	384	201	76	56	154
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CR00	387	300	79	59	388	201	80	60	158
CR00	388	300	80	60	389	201	81	61	159
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CR00	390	300	82	62	391	201	83	63	161
CR00	391	300	83	63	392	201	84	64	162
CR00	392	300	84	64	393	201	85	65	163
CR00	393	300	85	65	394	201	86	66	164
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CR00	409	300	101	81	410	201	102	82	180
CR00	410	300	102	82	411	201	103	83	181
CR00	411	300	103	83	412	201	104	84	182
CR00	412	300	104	84	413	201	105	85	183
CR00	413	300	105	85	414	201	106	86	184
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CR00	415	300	107	87	416	201	108	88	186
CR00	416	300	108	88	417	201	109	89	187
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CR00	418	300	110	90	419	201	111	91	189
CR00	419	300	111	91	420	201	112	92	190
CR00	420	300	112	92	421	201	113	93	191
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CR00	422	300	114	94	423	201	115	95	193
CR00	423	300	115	95	424	201	116	96	194
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CR00	455	300	147	127	456	201	148	128	226
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CR00	457	300	149	129	458	201	150	130	228
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CR00	475	300	167	147	476	201	168	148	246
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CR00	488	300	180	160	489	201	181	161	259
CR00	489	300	181	161	490	201	182	162	260
CR00	490	300	182	162	491	201	183	163	261
CR00	491	300	183	163	492	201	184	164	262
CR00	492	300	184	164	493	201	185	165	263
CR00	493	300	185	165	494	201	186	166	264
CR00	494	300	186	166	495	201	187	167	265
CR00	495	3							

FOR RODS BETWEEN FRONT SPAR AND LEADING EDGE AND FOR RODS BETWEEN SPARS, PANELS 1 THRU 5			FOR RODS IN BOTH SPARS			FOR RODS IN RIBS AND FOR ALL VERTICAL RODS			
PROB	NO	VALUE	PROB	NO	VALUE	PROB	NO	VALUE	
PROB	67	1.88	PROB	888200	21	PROB	888200	18	5.02
PROB	68	1.89	PROB	888201	22	PROB	888201	19	.001
PROB	69	1.70	PROB	888202	23	PROB	888202	20	.30
PROB	70	1.73	PROB	888203	24	PROB	888203	21	.50
PROB	71	1.74	PROB	888204	25	PROB	888204	22	1.02
PROB	72	1.75	PROB	888205	26	PROB	888205	23	1.02
PROB	73	1.76	PROB	888206	27	PROB	888206	24	1.02
PROB	74	1.78	PROB	888207	28	PROB	888207	25	1.02
PROB	75	1.79	PROB	888208	29	PROB	888208	26	1.02
PROB	76	1.80	PROB	888209	30	PROB	888209	27	1.02
PROB	77	1.81	PROB	888210	31	PROB	888210	28	1.02
PROB	78	1.82	PROB	888211	32	PROB	888211	29	1.02
PROB	79	1.83	PROB	888212	33	PROB	888212	30	1.02
PROB	80	1.84	PROB	888213	34	PROB	888213	31	1.02
PROB	81	1.85	PROB	888214	35	PROB	888214	32	1.02
PROB	82	1.86	PROB	888215	36	PROB	888215	33	1.02
PROB	83	1.87	PROB	888216	37	PROB	888216	34	1.02
PROB	84	1.88	PROB	888217	38	PROB	888217	35	1.02
PROB	85	1.89	PROB	888218	39	PROB	888218	36	1.02
PROB	86	1.90	PROB	888219	40	PROB	888219	37	1.02
PROB	87	1.91	PROB	888220	41	PROB	888220	38	1.02
PROB	88	1.92	PROB	888221	42	PROB	888221	39	1.02
PROB	89	1.93	PROB	888222	43	PROB	888222	40	1.02
PROB	90	1.94	PROB	888223	44	PROB	888223	41	1.02
PROB	91	1.95	PROB	888224	45	PROB	888224	42	1.02
PROB	92	1.96	PROB	888225	46	PROB	888225	43	1.02
PROB	93	1.97	PROB	888226	47	PROB	888226	44	1.02
PROB	94	1.98	PROB	888227	48	PROB	888227	45	1.02
PROB	95	1.99	PROB	888228	49	PROB	888228	46	1.02
PROB	96	2.00	PROB	888229	50	PROB	888229	47	1.02
PROB	97	2.01	PROB	888230	51	PROB	888230	48	1.02
PROB	98	2.02	PROB	888231	52	PROB	888231	49	1.02
PROB	99	2.03	PROB	888232	53	PROB	888232	50	1.02
PROB	100	2.04	PROB	888233	54	PROB	888233	51	1.02
PROB	101	2.05	PROB	888234	55	PROB	888234	52	1.02
PROB	102	2.06	PROB	888235	56	PROB	888235	53	1.02
PROB	103	2.07	PROB	888236	57	PROB	888236	54	1.02
PROB	104	2.08	PROB	888237	58	PROB	888237	55	1.02
PROB	105	2.09	PROB	888238	59	PROB	888238	56	1.02
PROB	106	2.10	PROB	888239	60	PROB	888239	57	1.02
PROB	107	2.11	PROB	888240	61	PROB	888240	58	1.02
PROB	108	2.12	PROB	888241	62	PROB	888241	59	1.02
PROB	109	2.13	PROB	888242	63	PROB	888242	60	1.02
PROB	110	2.14	PROB	888243	64	PROB	888243	61	1.02
PROB	111	2.15	PROB	888244	65	PROB	888244	62	1.02
PROB	112	2.16	PROB	888245	66	PROB	888245	63	1.02
PROB	113	2.17	PROB	888246	67	PROB	888246	64	1.02
PROB	114	2.18	PROB	888247	68	PROB	888247	65	1.02
PROB	115	2.19	PROB	888248	69	PROB	888248	66	1.02
PROB	116	2.20	PROB	888249	70	PROB	888249	67	1.02
PROB	117	2.21	PROB	888250	71	PROB	888250	68	1.02
PROB	118	2.22	PROB	888251	72	PROB	888251	69	1.02
PROB	119	2.23	PROB	888252	73	PROB	888252	70	1.02
PROB	120	2.24	PROB	888253	74	PROB	888253	71	1.02
PROB	121	2.25	PROB	888254	75	PROB	888254	72	1.02
PROB	122	2.26	PROB	888255	76	PROB	888255	73	1.02
PROB	123	2.27	PROB	888256	77	PROB	888256	74	1.02
PROB	124	2.28	PROB	888257	78	PROB	888257	75	1.02
PROB	125	2.29	PROB	888258	79	PROB	888258	76	1.02
PROB	126	2.30	PROB	888259	80	PROB	888259	77	1.02
PROB	127	2.31	PROB	888260	81	PROB	888260	78	1.02
PROB	128	2.32	PROB	888261	82	PROB	888261	79	1.02
PROB	129	2.33	PROB	888262	83	PROB	888262	80	1.02
PROB	130	2.34	PROB	888263	84	PROB	888263	81	1.02
PROB	131	2.35	PROB	888264	85	PROB	888264	82	1.02
PROB	132	2.36	PROB	888265	86	PROB	888265	83	1.02
PROB	133	2.37	PROB	888266	87	PROB	888266	84	1.02
PROB	134	2.38	PROB	888267	88	PROB	888267	85	1.02
PROB	135	2.39	PROB	888268	89	PROB	888268	86	1.02
PROB	136	2.40	PROB	888269	90	PROB	888269	87	1.02
PROB	137	2.41	PROB	888270	91	PROB	888270	88	1.02
PROB	138	2.42	PROB	888271	92	PROB	888271	89	1.02
PROB	139	2.43	PROB	888272	93	PROB	888272	90	1.02
PROB	140	2.44	PROB	888273	94	PROB	888273	91	1.02
PROB	141	2.45	PROB	888274	95	PROB	888274	92	1.02
PROB	142	2.46	PROB	888275	96	PROB	888275	93	1.02
PROB	143	2.47	PROB	888276	97	PROB	888276	94	1.02
PROB	144	2.48	PROB	888277	98	PROB	888277	95	1.02
PROB	145	2.49	PROB	888278	99	PROB	888278	96	1.02
PROB	146	2.50	PROB	888279	100	PROB	888279	97	1.02
PROB	147	2.51	PROB	888280	101	PROB	888280	98	1.02
PROB	148	2.52	PROB	888281	102	PROB	888281	99	1.02
PROB	149	2.53	PROB	888282	103	PROB	888282	100	1.02
PROB	150	2.54	PROB	888283	104	PROB	888283	101	1.02
PROB	151	2.55	PROB	888284	105	PROB	888284	102	1.02
PROB	152	2.56	PROB	888285	106	PROB	888285	103	1.02
PROB	153	2.57	PROB	888286	107	PROB	888286	104	1.02
PROB	154	2.58	PROB	888287	108	PROB	888287	105	1.02
PROB	155	2.59	PROB	888288	109	PROB	888288	106	1.02
PROB	156	2.60	PROB	888289	110	PROB	888289	107	1.02
PROB	157	2.61	PROB	888290	111	PROB	888290	108	1.02
PROB	158	2.62	PROB	888291	112	PROB	888291	109	1.02
PROB	159	2.63	PROB	888292	113	PROB	888292	110	1.02
PROB	160	2.64	PROB	888293	114	PROB	888293	111	1.02
PROB	161	2.65	PROB	888294	115	PROB	888294	112	1.02
PROB	162	2.66	PROB	888295	116	PROB	888295	113	1.02
PROB	163	2.67	PROB	888296	117	PROB	888296	114	1.02
PROB	164	2.68	PROB	888297	118	PROB	888297	115	1.02
PROB	165	2.69	PROB	888298	119	PROB	888298	116	1.02
PROB	166	2.70	PROB	888299	120	PROB	888299	117	1.02
PROB	167	2.71	PROB	888300	121	PROB	888300	118	1.02
PROB	168	2.72	PROB	888301	122	PROB	888301	119	1.02
PROB	169	2.73	PROB	888302	123	PROB	888302	120	1.02
PROB	170	2.74	PROB	888303	124	PROB	888303	121	1.02
PROB	171	2.75	PROB	888304	125	PROB	888304	122	1.02
PROB	172	2.76	PROB	888305	126	PROB	888305	123	1.02
PROB	173	2.77	PROB	888306	127	PROB	888306	124	1.02
PROB	174	2.78	PROB	888307	128	PROB	888307	125	1.02
PROB	175	2.79	PROB	888308	129	PROB	888308	126	1.02
PROB	176	2.80	PROB	888309	130	PROB	888309	127	1.02
PROB	177	2.81	PROB	888310	131	PROB	888310	128	1.02
PROB	178	2.82	PROB	888311	132	PROB	888311	129	1.02
PROB	179	2.83	PROB	888312	133	PROB	888312	130	1.02
PROB	180	2.84	PROB	888313	134	PROB	888313	131	1.02
PROB	181	2.85	PROB	888314	135	PROB	888314	132	1.02
PROB	182	2.86	PROB	888315	136	PROB	888315	133	1.02
PROB	183	2.87	PROB	888316	137	PROB	888316	134	1.02
PROB	184	2.88	PROB	888317	138	PROB	888317	135	1.02
PROB	185	2.89	PROB	888318	139	PROB	888318	136	1.02
PROB	186	2.90	PROB	888319	140	PROB	888319	137	1.02
PROB	187	2.91	PROB	888320	141	PROB	888320	138	1.02
PROB	188	2.92	PROB	888321	142	PROB	888321	139	1.02
PROB	189	2.93	PROB	888322	143	PROB	888322	140	1.02
PROB	190	2.94	PROB	888323	144	PROB	888323	141	1.02
PROB	191	2.95	PROB	888324	145	PROB	888324	142	1.02
PROB	192	2.96	PROB	888325	146	PROB	888325	143	1.02
PROB	193	2.97	PROB	888326	147	PROB	888326	144	1.02
PROB	194	2.98	PROB	888327	148	PROB	888327	145	1.02
PROB	195	2.99	PROB	888328	149	PROB	888328	146	1.02
PROB	196	3.00	PROB	888329	150	PROB	888329	147	1.02
PROB	197	3.01	PROB	888330	151	PROB	888330	148	1.02
PROB	198	3.02	PROB	888331	152	PROB	888331	149	1.02
PROB	199	3.03	PROB	888332	153	PROB	888332	150	1.02
PROB	200	3.04	PROB	888333	154	PROB	888333	151	1.02
PROB	201	3.05	PROB	888334	155	PROB	888334	152	1.02
PROB	202	3.06	PROB	888335	156	PROB	888335	153	1.02
PROB	203	3.07	PROB	888336	157	PROB	88833		

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LOAD	102	1.0	2.0			
LOAD	103	1.0	3.0			
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MET LEASTING CONDITIONS						
LOAD	201	1.0	1.0			
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LOAD	244	1.0	44.0			
LOAD	245	1.0	45.0			
END DATA						

APPENDIX F

FINITE ELEMENT TORSIONAL ROD LISTING

[illegible]

APPENDIX G

FINITE ELEMENT INITIAL DAMAGE MODELING

[illegible]

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SS ..... BULK DATA DECK FOR TEST 2C .....
SS ..... F-BAR WING PROJECT - DAMAGED WING .....
SS ..... MODEL A - STATIC CASE .....
SS ..... SPARS AND RIBS CROD AND CSHEAR .....
SS ..... STIFFENERS CROD .....
SS ..... SKIN PANELS CROD .....
SS ..... NO TORSIONAL STIFFNESS IN SPARS .....
SS .....
SS ..... SHEAR PANELS - TOP OF WING .....
SS .....
SS ..... CHANGE CSHEAR 83 NEW PID-9999 OLD PID-222252 .....
SS ..... CHANGE CSHEAR 84 NEW PID-11112 OLD PID-2 .....
SS .....
SS ..... VERTICAL SHEAR PANELS .....
SS .....
SS ..... CHANGE CSHEAR 237 NEW PID-9999 OLD PID-2222401 .....
SS .....
SS ..... HORIZONTAL ROOFS .....
SS .....
SS ..... CHANGE CROD 509 NEW PID-9999 OLD PID-416 .....
SS ..... CHANGE CROD 510 NEW PID-9999 OLD PID-417 .....
SS ..... CHANGE CROD 511 NEW PID-8888402 OLD PID-409 .....
SS .....
SS ..... SPECIAL PROPERTY AND MATERIAL CARDS FOR INITIAL DAMAGE .....
SS .....
SS ..... MAT1 99 10.0 5.0 .....
SS ..... PROO 99.9+9 99.9+9 99.9+9 .....
SS ..... 3.14-2 .....
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SS ..... BULK DATA DECK FOR TEST 2C .....
SS ..... F-BAR WING PROJECT - DAMAGED WING .....
SS ..... MODEL C - STATIC CASE .....
SS ..... SPARS AND RIBS CROD AND CSHEAR .....
SS ..... STIFFENERS CROD .....
SS ..... SKIN PANELS CROD .....
SS ..... NO TORSIONAL STIFFNESS IN SPARS .....
SS .....
SS ..... CORNER PANELS - TOP OF WING .....
SS .....
SS ..... CHANGE CORNER 83 NEW PID-9999 OLD PID-222252 .....
SS ..... CHANGE CORNER 84 NEW PID-11112 OLD PID-2 .....
SS .....
SS ..... VERTICAL SHEAR PANELS .....
SS .....
SS ..... CHANGE CSHEAR 237 NEW PID-9999 OLD PID-2222401 .....
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SS ..... HORIZONTAL ROOFS .....
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SS ..... CHANGE CROD 509 NEW PID-9999 OLD PID-423 .....
SS ..... CHANGE CROD 510 NEW PID-9999 OLD PID-515 .....
SS ..... CHANGE CROD 511 NEW PID-8888901 OLD PID-506 .....
SS .....
SS ..... SPECIAL PROPERTY AND MATERIAL CARDS FOR INITIAL DAMAGE .....
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SS ..... MAT1 99 10.0 5.0 .....
SS ..... PROO 99.9+9 99.9+9 99.9+9 .....
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APPENDIX H

PROGRESSIVE STRUCTURAL COLLAPSE ANALYSIS LISTING

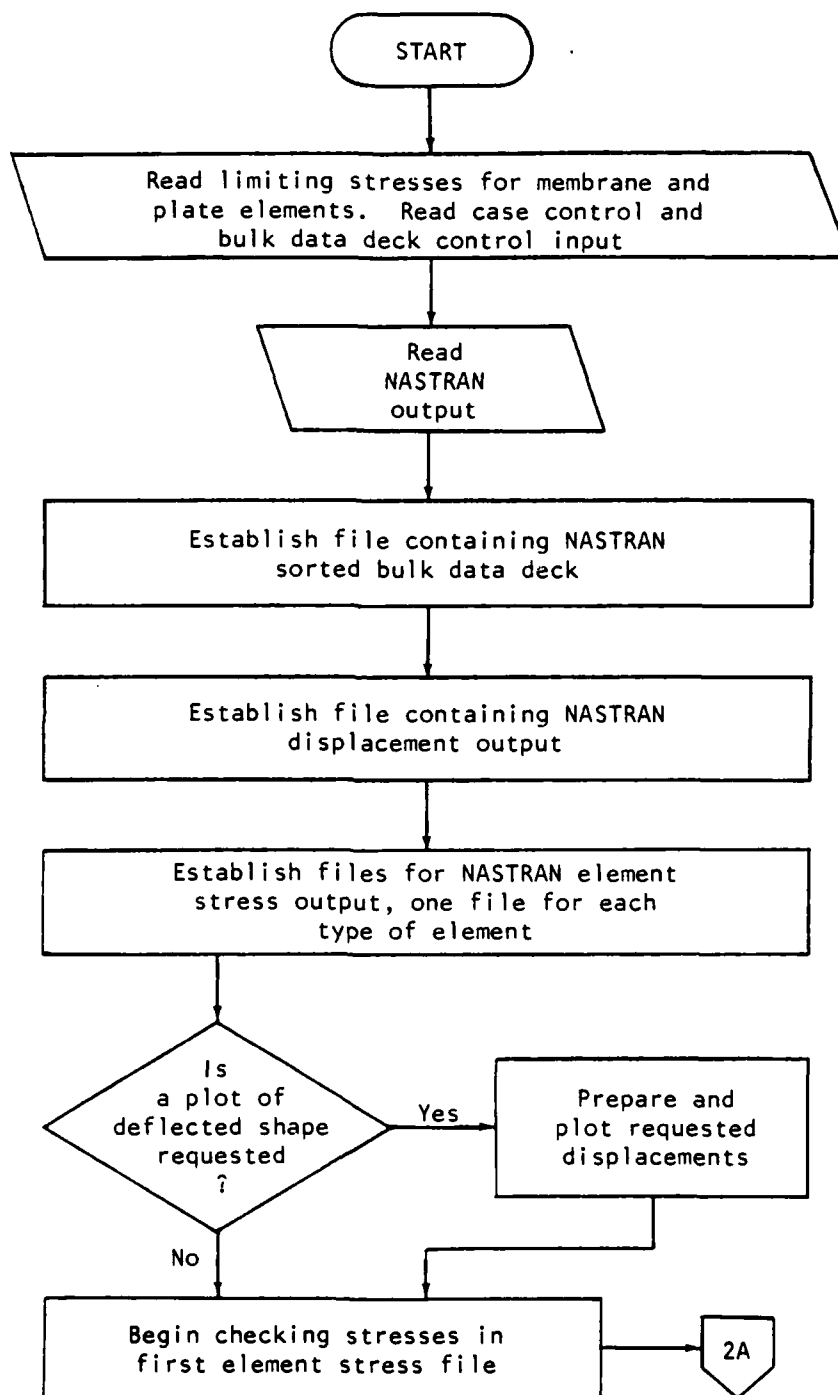


Figure 25. PROSCAN Functional Flow Diagram

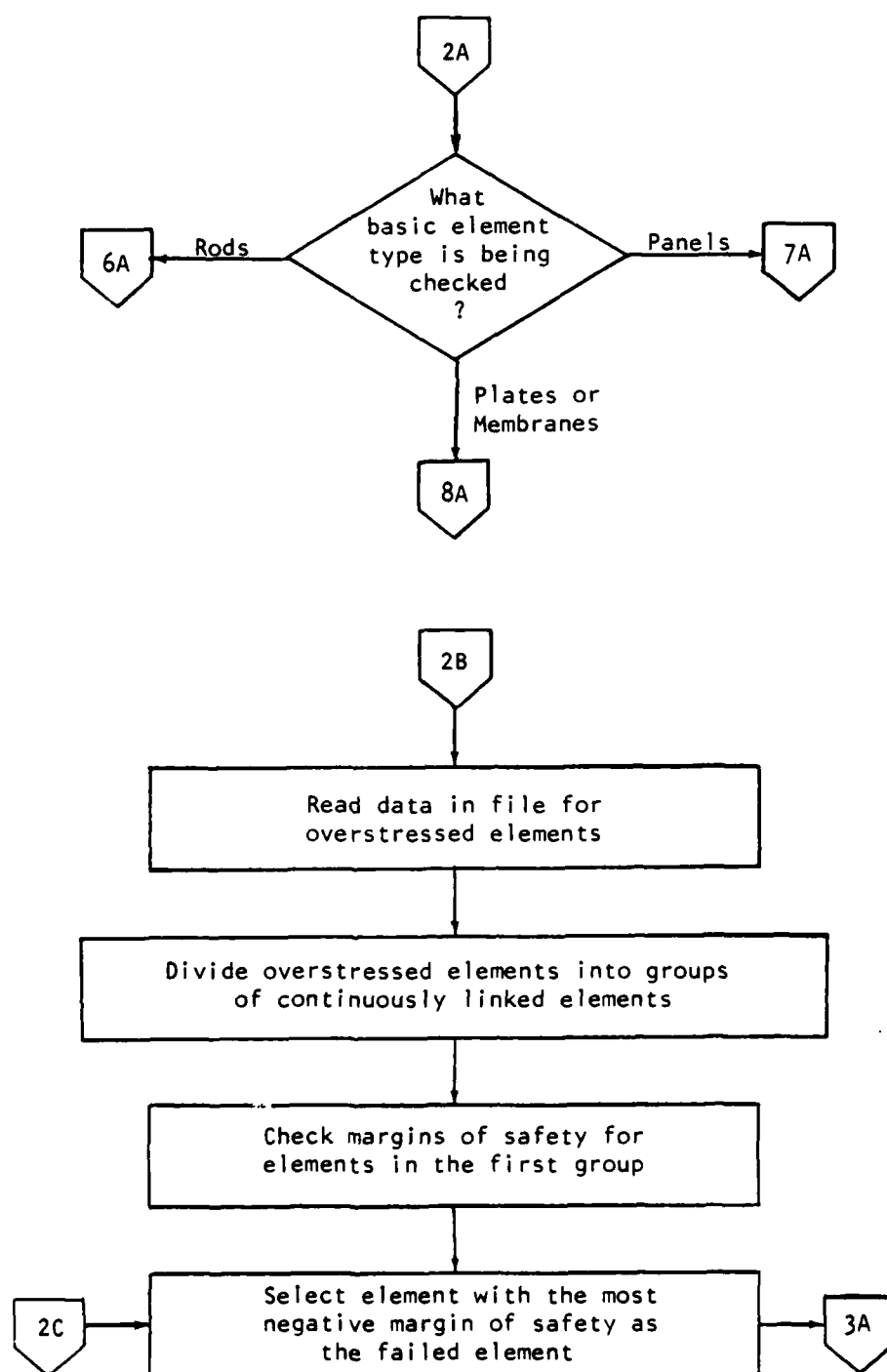


Figure 25. (Continued)

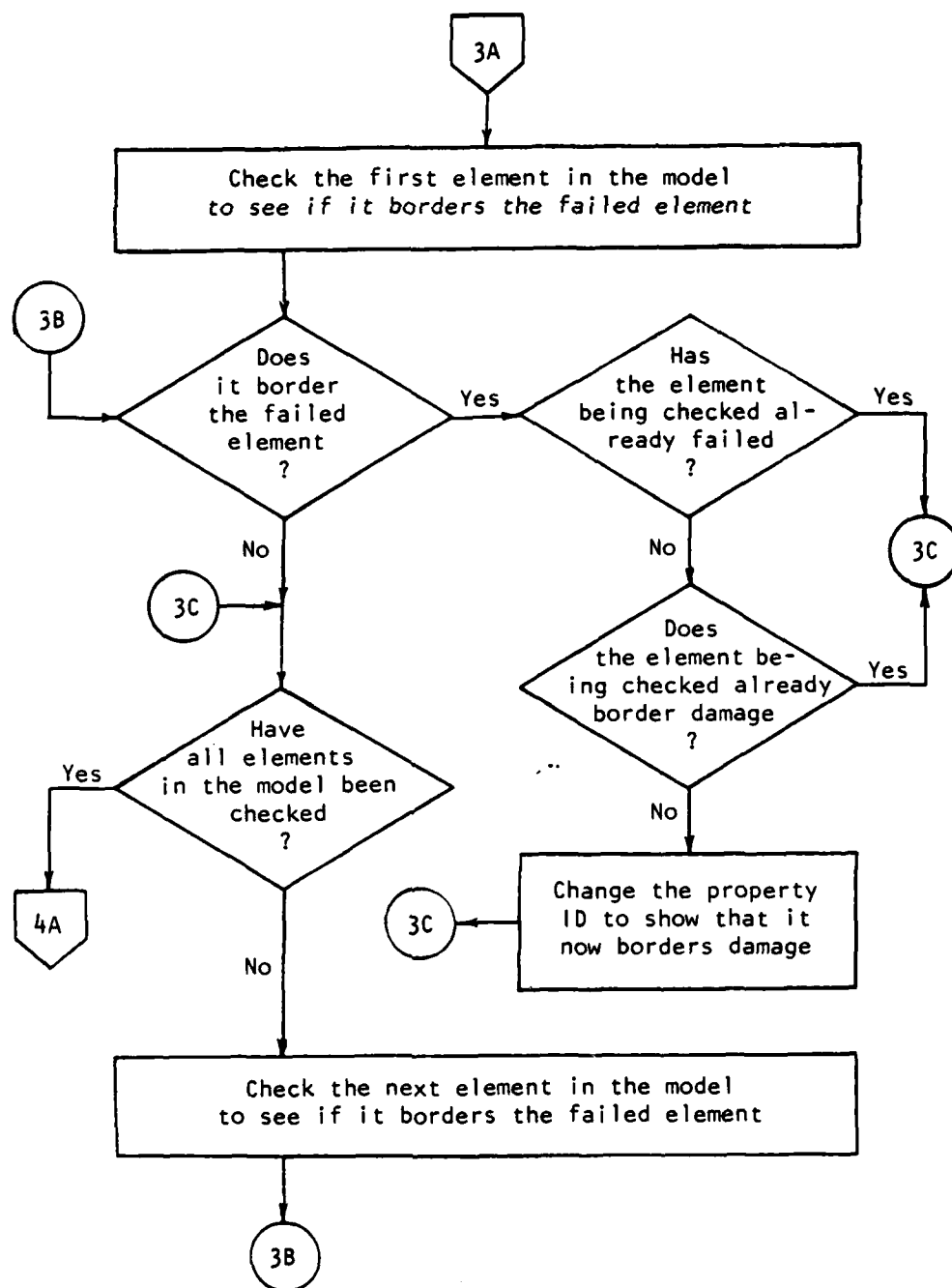


Figure 25. (Continued)

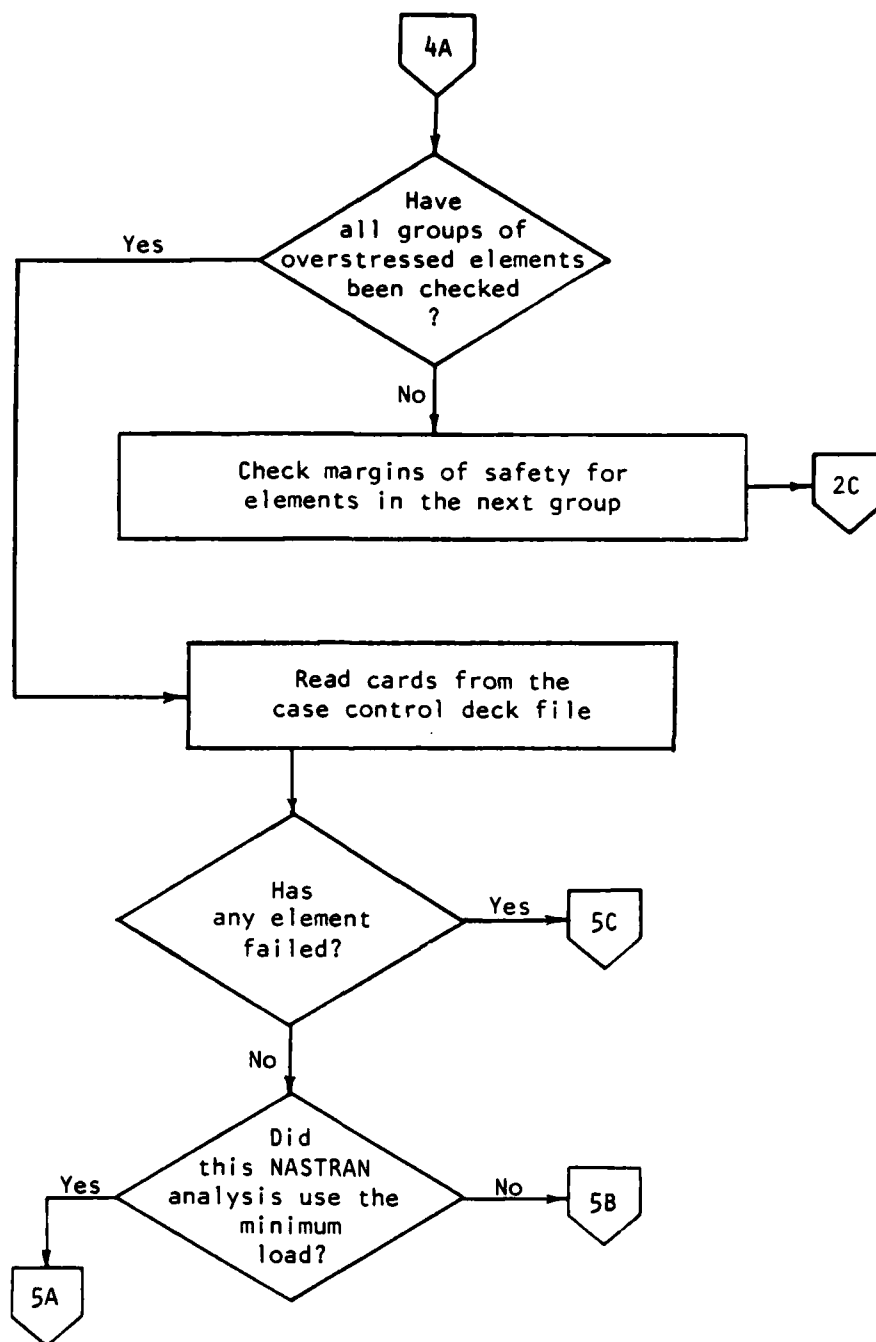


Figure 25. (Continued)

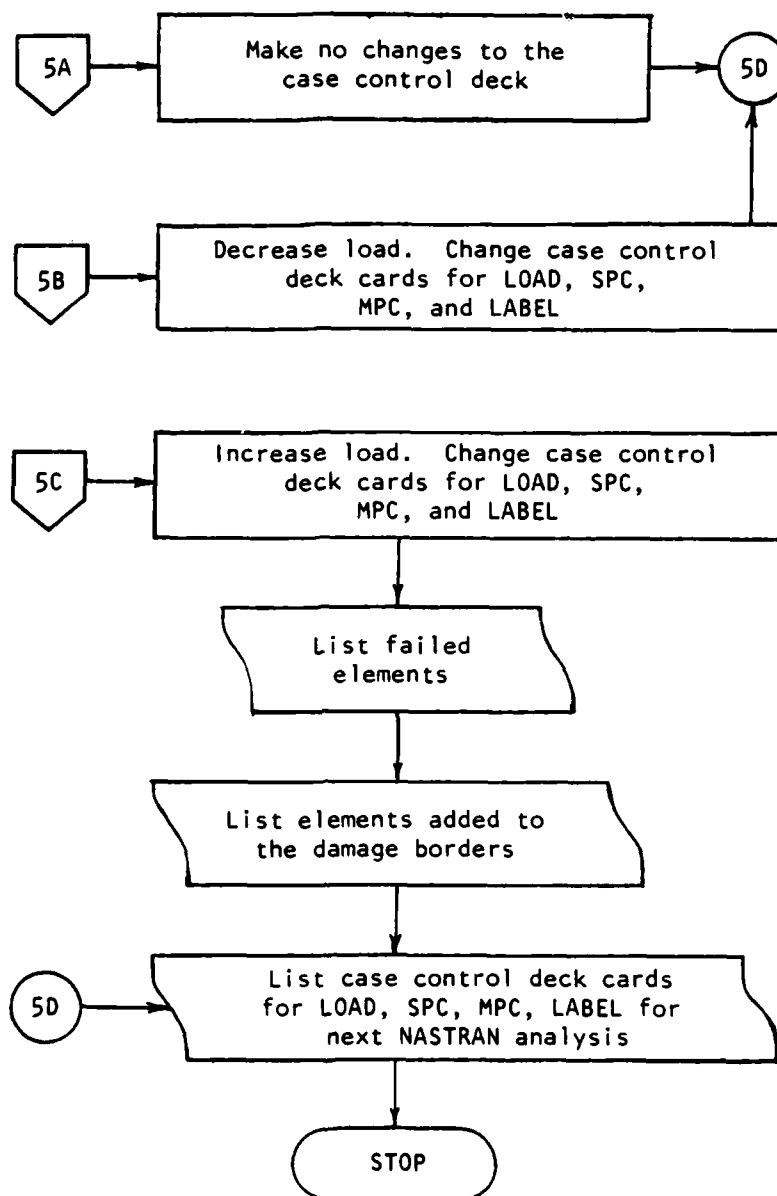


Figure 25. (Continued)

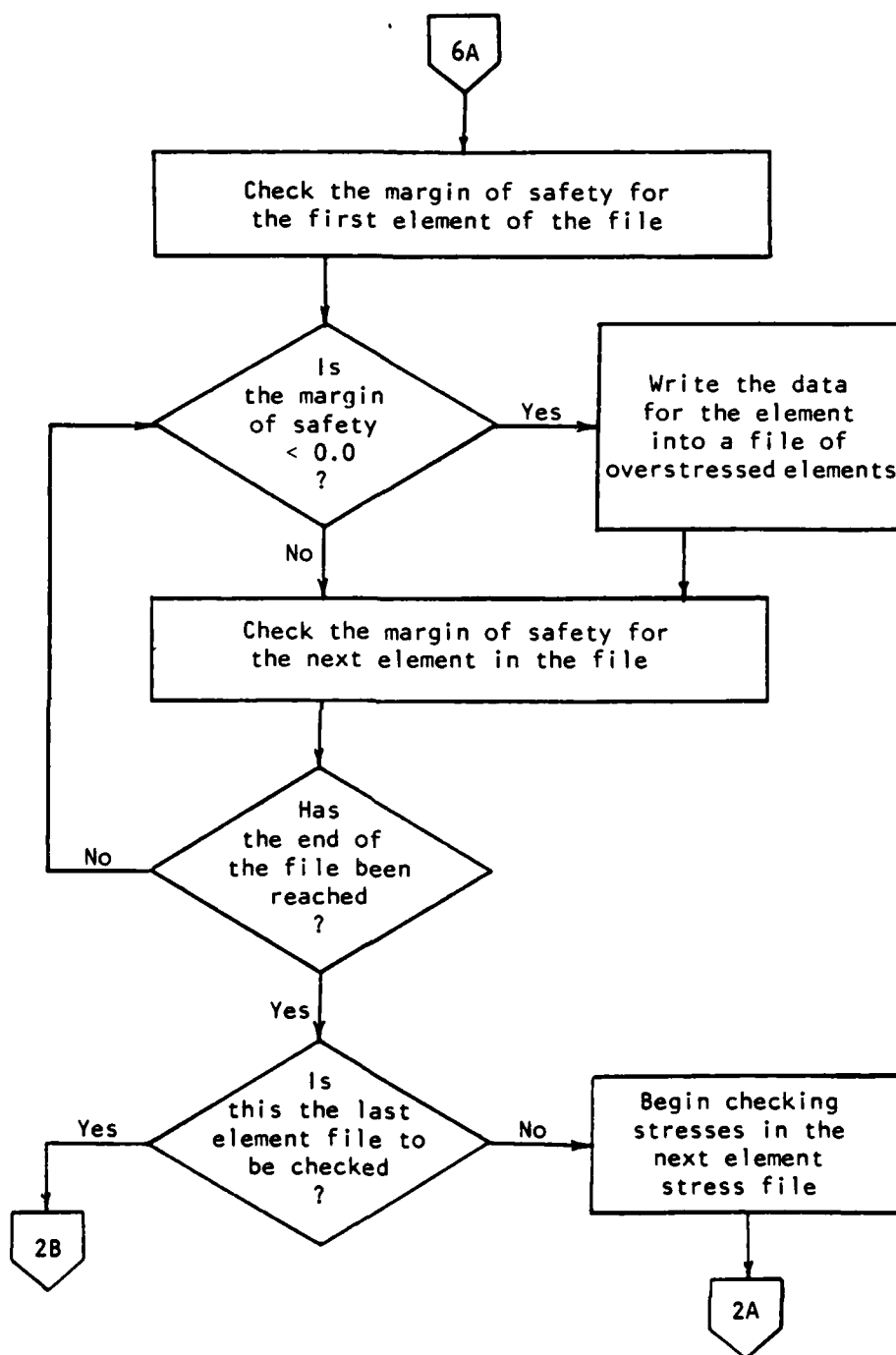


Figure 25. (Continued)

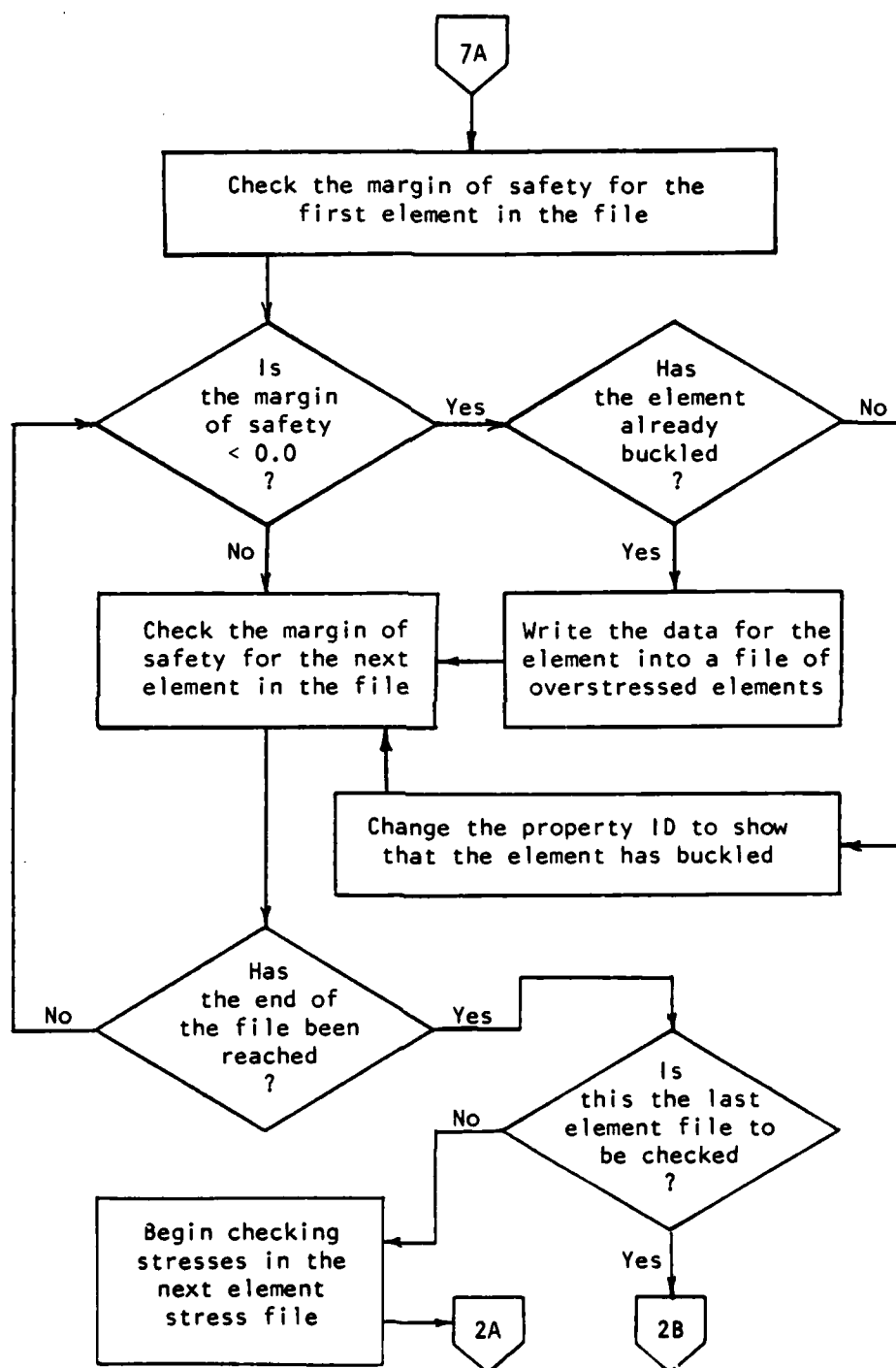


Figure 25. (Continued)

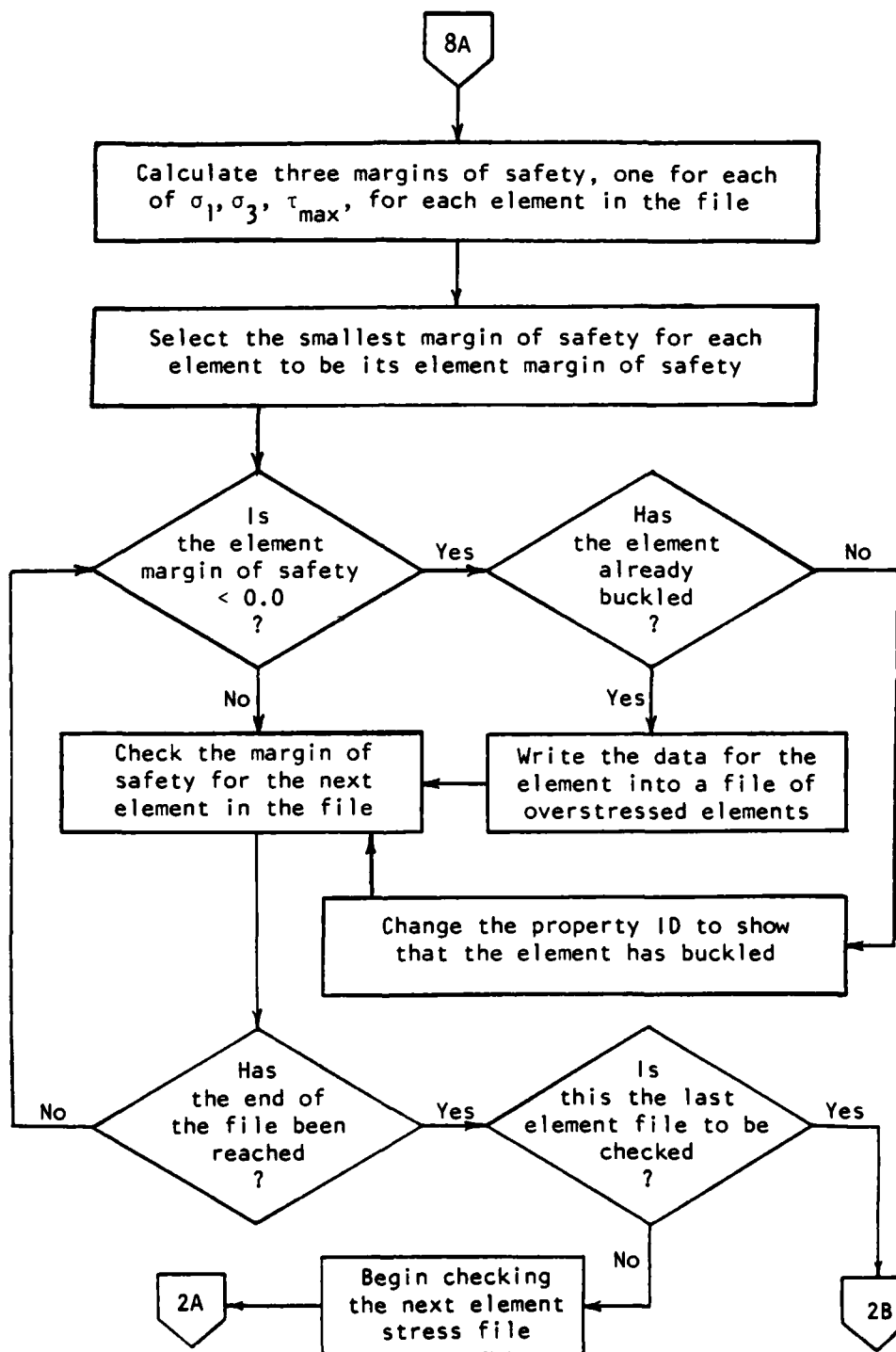


Figure 25. (Continued)

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00014690 IF ( IPSW .GT. 4 ) PRINT 9020, ELTYPE, ELNUM, ITT, CARD
00014695 IF ( CARD(1) .EQ. ELTYPE(1) .AND. CARD(2) .EQ. ELTYPE(2) )
00014700 *GO TO 380
00014705 WRITE (LUN2,1000) CARD
00014710
00014715 380 CONTINUE
00014720
00014725 BACKSPACE LUN1
00014730 IF ( ELTYPE(1) .EQ. CNOO .OR. ELTYPE(1) .EQ. CTUB ) GO TO 460
00014735 IF ( ELTYPE(1) .EQ. CON .AND. ELTYPE(2) .EQ. ROD ) GO TO 520
00014740
00014745 C CHECK ELEMENTS WITH THREE OR FOUR NODES
00014750
00014755 400 CONTINUE (LUN1,1000,END=950) CARD
00014760 READ ITT = 2
00014765 IF ( IPSW .GT. 4 ) PRINT 9030, ELTYPE, ELNUM, ITT, CARD
00014770 IF ( CARD(3) .EQ. ELNUM(1) .AND. CARD(4) .EQ. ELNUM(2) )
00014775 *GO TO 420
00014780 WRITE (LUN2,1000) CARD
00014785 GO TO 400
00014790 IF ( CARD(1) .L= 1, 4 ) .CARD(3), CARD(4) .PIOF
00014795 420 PRINT 9900, CARD(3) = PIOF
00014800 CARD(3) = BLANK
00014805 WRITE (LUN2,1000) CARD
00014810
00014815 440 GO TO 540
00014820
00014825 C CHECK GRID AND CTUBE ELEMENTS
00014830
00014835 460 CONTINUE (LUN1,1000,END=950) CARD
00014840 READ ITT = 3
00014845 IF ( IPSW .GT. 4 ) PRINT 9030, ELTYPE, ELNUM, ITT, CARD
00014850 IF ( CARD(3) .EQ. ELNUM(1) .AND. CARD(4) .EQ. ELNUM(2) )
00014855 *GO TO 500
00014860 IF ( CARD(11) .EQ. ELNUM(1) .AND. CARD(12) .EQ. ELNUM(2) )
00014865 *GO TO 520
00014870 WRITE (LUN2,1000) CARD
00014875 GO TO 460
00014880 500 PRINT 9901, ( CARD(L), L=1, 4 ) .CARD(3), CARD(4) .PIOF
00014885 CARD(3) = PIOF
00014890 CARD(3) = BLANK
00014895 WRITE (LUN2,1000) CARD
00014900 GO TO 540
00014905 520 PRINT 9901, CARD(1), CARD(2), (CARD(L), L=1, 14) .PIOF
00014910 CARD(12) = PIOF
00014915 CARD(12) = BLANK
00014920 WRITE (LUN2,1000) CARD
00014925 JFLAG = 3
00014930
00014935 540 GO TO 540
00014940
00014945 C CHECK CORROD ELEMENTS
00014950
00014955 525 CONTINUE
00014960 READ (LUN1,1000,END=950) CARD
00014965 IF ( CARD(3) .EQ. ELNUM(1) .AND. CARD(4) .EQ. ELNUM(2) )
00014970 *GO TO 530
00014975 WRITE (LUN2,1000) CARD
00014980 GO TO 525
00014985 530 CONTINUE
00014990 PRINT 9902, ( CARD(L), L=1, 4 ) .CARD(3), CARD(4) .JWIDF
00014995 CARD(3) = JWIDF
00015000 CARD(10) = BLANK
00015005 WRITE (LUN2,1000) CARD
00015010
00015015 540 CONTINUE
00015020
00015025 C CALL SUBROUTINE GPFALL TO ADD THE NODES OF THE FAILED ELEMENT
00015030
00015035 CALL GPFALL ( JFLAG )
00015040 IF ( IPSW .GT. 3 ) PRINT 9012
00015045 IF ( IPSW .GT. 3 ) PRINT 9014
00015050 READ (LUN1,1000,END=950) CARD
00015055 WRITE (LUN2,1000) CARD
00015060 GO TO 530
00015065 560 CONTINUE
00015070
00015075 REWIND LUNP
00015080 REWIND LUN1
00015085 REWIND LUN2
00015090 THOLD = LUN1
00015095 LUN1 = LUN2
00015100
00015105
00015110
00015115
00015120
00015125
00015130
00015135
00015140
00015145
00015150
00015155
00015160
00015165
00015170
00015175
00015180
00015185
00015190
00015195
00015200

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00013890 IF ( IPSW .GT. 3 ) PRINT 9011
00013895 CALL ELNODE ( ELTP2(1), ELTP2(2), ELNUM(1), ELNUM(2), NW, NK, IPSW )
00013900 IF ( IPSW .GT. 3 ) PRINT 9012
00013905 DO 260 L = 1, N
00013910 DO 240 M = 1, NM
00013915 *GO IF ( GP(1,L) .EQ. GP(2,L) .AND. GP(2,L) .EQ. GP(2,M) )
00013920 GO TO 240
00013925 200 CONTINUE
00013930
00013935 MFLAG2 = MAMPAS
00013940 MFAIL = MFAIL + 1
00013945 JSWITCH = 1
00013950 WRITE (LUNP,4000) ELTP2, ELNUM2, GP2, NCHK, MFLAG2, EMDS2
00013955 IF ( IPSW .GT. 2 ) PRINT 9006, ELTP2, ELNUM2, GP2,
00013960 NCHK, MFLAG2, EMDS2
00013965 *GO IF ( IPSW .GT. 4 ) PRINT 9008, GP2, NCHK, MFLAG2, EMDS2
00013970 IF ( IPSW .GT. 4 ) PRINT 9008, GP2, NCHK, MFLAG2, EMDS2
00013975
00013980 GO TO 260
00013985 240 CONTINUE
00013990
00013995 260 CONTINUE
00014000
00014005 WRITE (LUNP,4000) ELTP2, ELNUM2, GP2, NCHK, MFLAG2, EMDS2
00014010 IF ( IPSW .GT. 4 ) PRINT 9008, GP2, NCHK, MFLAG2, EMDS2
00014015
00014020 280 CONTINUE
00014025
00014030 THOLD = LUNP1
00014035 LUNP2 = LUNP2
00014040 LUNP3 = LUNP3
00014045 INHOLD = IF2
00014050 IF1 = IF2
00014055 IF2 = INHOLD
00014060 IF ( IPSW .GT. 5 ) PRINT 9905, LUNP1, LUNP2, IF1, IF2
00014065 REWIND LUNP1
00014070 REWIND LUN2
00014075 DO 290 NM = 1, 1
00014080 READ (LUNP1,1000,END=600) CARD
00014085 IF ( IPSW .GT. 2 ) PRINT 9010, IF1, CARD
00014090 290 CONTINUE
00014095
00014100 SWITCH .EQ. 0 ) GO TO 480
00014105 IF ( SWITCH .EQ. 0 ) GO TO 310
00014110 REWIND LUNP1
00014115 GO TO 105
00014120
00014125 C SELECT MOST SEVERELY STRESSED OF THE GROUPED ELEMENTS
00014130
00014135 REWIND LUNP2
00014140 REWIND LUNP3
00014145 REWIND LUNP
00014150 ELTYPE(1) = BLANK
00014155 ELTYPE(2) = BLANK
00014160 ELNUM(1) = BLANK
00014165 ELNUM(2) = BLANK
00014170 FAIL = 0.0
00014175 DO 320 I = 1, MFAIL
00014180 READ (LUNP,4000) ELTP1, ELNUM1, GP1, NCHK, MFLAG1, EMDS1
00014185 IF ( IPSW .GT. 4 ) PRINT 9009, ELTP1, ELNUM1, GP1,
00014190 NCHK, MFLAG1, EMDS1
00014195 IF ( ELNUM1 .GT. 240 ) GO TO 340
00014200 IF ( ELNUM1 .GT. 240 ) GO TO 340
00014205 EMDS IS THE "ELEMENT MARGIN OF SAFETY"
00014210
00014215 320 CONTINUE
00014220
00014225 340 CONTINUE
00014230
00014235 ELNUM(1) = ELNUM(1)
00014240 ELNUM(2) = ELNUM(2)
00014245 ELTYPE(1) = ELTYPE(1)
00014250 ELTYPE(2) = ELTYPE(2)
00014255 FAIL = EMDS1
00014260 JFLAG = 1
00014265
00014270 GO TO 320
00014275
00014280 C CHANGE APPROPRIATE ELEMENT PROPERTY ID
00014285
00014290 360 CONTINUE
00014295 ITT = 1
00014300 READ (LUN1,1000,END=950) CARD
00014305
00014310
00014315
00014320
00014325
00014330
00014335
00014340
00014345
00014350
00014355
00014360
00014365
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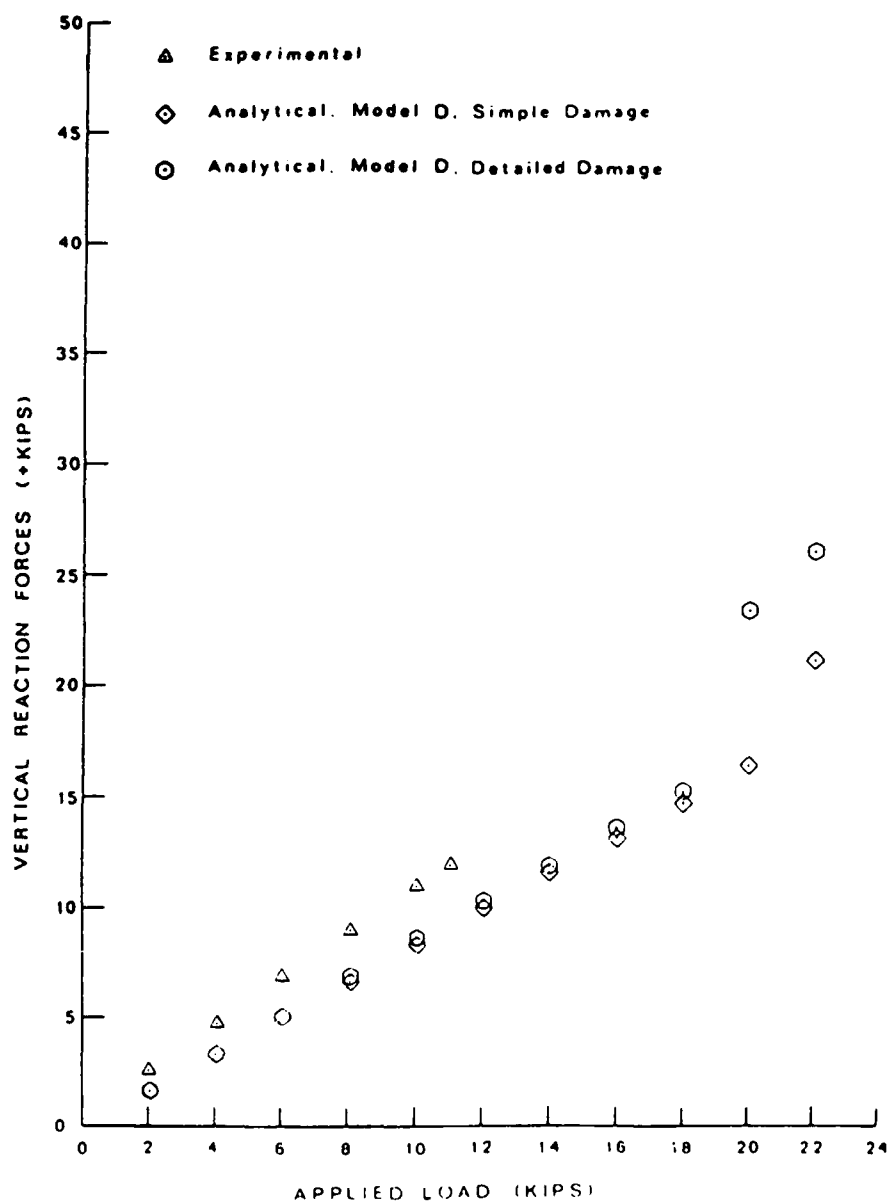
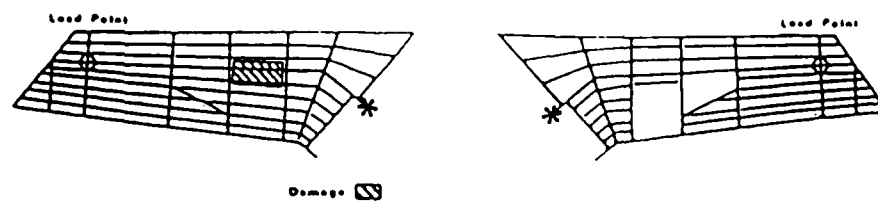

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00024300      IF ( MPPL EQ 0 ) GO TO 20
00024310      VMIN = V(1)
00024320      VMAX = V(1)
00024330      DO 10 I = 2, MPPL
00024340          VMIN = MIN( VMIN, V(I) )
00024350          VMAX = MAX( VMAX, V(I) )
00024360
00024370      10 CONTINUE
00024380
00024390      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00024400      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00024410      VINC = (VMAX-VMIN) / MG
00024420      I = 1
00024430      10 CONTINUE
00024440      IF ( A .LT. 0.0 ) I = 1 - I
00024450      SCALE = 10.0 ** ( MD - I )
00024460      VINC = VINC * SCALE
00024470
00024480      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00024490      DO 30 I = 1, N
00024500          IF ( A GE VINC ) GO TO 50
00024510          30 CONTINUE
00024520      I = 1
00024530      40 CONTINUE
00024540      SCALE = SCALE / 10.0
00024550      50 CONTINUE
00024560      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00024570      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00024580      VINC = A / SCALE
00024590      A = VMIN / VINC
00024600      IF ( A .LT. 0.0 ) J = J - 1
00024610      SHIN = J * VINC
00024620      SMAX = SHIN + VINC = MG
00024630      IF ( SMAX GE VMAX ) GO TO 60
00024640      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00024650      IF ( I GE 99 ) GO TO 40
00024660      I = I + 1
00024670      60 RETURN
00024680      END
00024690
00024700      SUBROUTINE PLT20A ( A, V, SHAR, INHIN, VMAX, VMIN )
00024710
00024720      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00024730      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00024740      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00024750
00024760      A = ARRAY OF X COORDINATES
00024770      MPPL = NUMBER OF POINTS IN X AND Y
00024780      VMIN = MINIMUM X VALUE
00024790      VMAX = MAXIMUM X VALUE
00024800      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00024810      AND MAXIMUM VALUES
00024820      VMIN = MINIMUM Y VALUE
00024830      VMAX = MAXIMUM Y VALUE
00024840      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00024850      AND MAXIMUM VALUES
00024860
00024870      COMMON / TEMPT / IPSW
00024880      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00024890      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00024900      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00024910      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00024920      IF ( IPSW GT 0 ) PRINT 9001
00024930      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20A FROM SUBROUTINE PLOT *****' )
00024940      LINE = 5
00024950
00024960      IF ( A EQ 0 ) GO TO 20
00024970      VMIN = V(1)
00024980      VMAX = V(1)
00024990      DO 10 I = 2, MPPL
00025000          VMIN = MIN( VMIN, V(I) )
00025010          VMAX = MAX( VMAX, V(I) )
00025020
00025030      10 CONTINUE
00025040
00025050      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00025060      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00025070      VINC = (VMAX-VMIN) / MG
00025080      I = 1
00025090      10 CONTINUE
00025100      IF ( A .LT. 0.0 ) I = 1 - I
00025110      SCALE = 10.0 ** ( MD - I )
00025120      VINC = VINC * SCALE
00025130
00025140      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00025150      DO 30 I = 1, N
00025160          IF ( A GE VINC ) GO TO 50
00025170          30 CONTINUE
00025180      I = 1
00025190      40 CONTINUE
00025200      SCALE = SCALE / 10.0
00025210      50 CONTINUE
00025220      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00025230      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00025240      VINC = A / SCALE
00025250      A = VMIN / VINC
00025260      IF ( A .LT. 0.0 ) J = J - 1
00025270      SHIN = J * VINC
00025280      SMAX = SHIN + VINC = MG
00025290      IF ( SMAX GE VMAX ) GO TO 60
00025300      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00025310      IF ( I GE 99 ) GO TO 40
00025320      I = I + 1
00025330      60 RETURN
00025340      END
00025350
00025360      SUBROUTINE PLT20B ( A, V, SHAR, INHIN, VMAX, VMIN )
00025370
00025380      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00025390      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00025400      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00025410
00025420      A = ARRAY OF X COORDINATES
00025430      MPPL = NUMBER OF POINTS IN X AND Y
00025440      VMIN = MINIMUM X VALUE
00025450      VMAX = MAXIMUM X VALUE
00025460      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00025470      AND MAXIMUM VALUES
00025480      VMIN = MINIMUM Y VALUE
00025490      VMAX = MAXIMUM Y VALUE
00025500      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00025510      AND MAXIMUM VALUES
00025520
00025530      COMMON / TEMPT / IPSW
00025540      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00025550      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00025560      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00025570      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00025580      IF ( IPSW GT 0 ) PRINT 9001
00025590      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20B FROM SUBROUTINE PLOT *****' )
00025600      LINE = 5
00025610
00025620      IF ( A EQ 0 ) GO TO 20
00025630      VMIN = V(1)
00025640      VMAX = V(1)
00025650      DO 10 I = 2, MPPL
00025660          VMIN = MIN( VMIN, V(I) )
00025670          VMAX = MAX( VMAX, V(I) )
00025680
00025690      10 CONTINUE
00025700
00025710      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00025720      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00025730      VINC = (VMAX-VMIN) / MG
00025740      I = 1
00025750      10 CONTINUE
00025760      IF ( A .LT. 0.0 ) I = 1 - I
00025770      SCALE = 10.0 ** ( MD - I )
00025780      VINC = VINC * SCALE
00025790
00025800      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00025810      DO 30 I = 1, N
00025820          IF ( A GE VINC ) GO TO 50
00025830          30 CONTINUE
00025840      I = 1
00025850      40 CONTINUE
00025860      SCALE = SCALE / 10.0
00025870      50 CONTINUE
00025880      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00025890      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00025900      VINC = A / SCALE
00025910      A = VMIN / VINC
00025920      IF ( A .LT. 0.0 ) J = J - 1
00025930      SHIN = J * VINC
00025940      SMAX = SHIN + VINC = MG
00025950      IF ( SMAX GE VMAX ) GO TO 60
00025960      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00025970      IF ( I GE 99 ) GO TO 40
00025980      I = I + 1
00025990      60 RETURN
00026000      END
00026010
00026020      SUBROUTINE PLT20C ( A, V, SHAR, INHIN, VMAX, VMIN )
00026030
00026040      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00026050      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00026060      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00026070
00026080      A = ARRAY OF X COORDINATES
00026090      MPPL = NUMBER OF POINTS IN X AND Y
00026100      VMIN = MINIMUM X VALUE
00026110      VMAX = MAXIMUM X VALUE
00026120      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00026130      AND MAXIMUM VALUES
00026140      VMIN = MINIMUM Y VALUE
00026150      VMAX = MAXIMUM Y VALUE
00026160      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00026170      AND MAXIMUM VALUES
00026180
00026190      COMMON / TEMPT / IPSW
00026200      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00026210      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00026220      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00026230      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00026240      IF ( IPSW GT 0 ) PRINT 9001
00026250      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20C FROM SUBROUTINE PLOT *****' )
00026260      LINE = 5
00026270
00026280      IF ( A EQ 0 ) GO TO 20
00026290      VMIN = V(1)
00026300      VMAX = V(1)
00026310      DO 10 I = 2, MPPL
00026320          VMIN = MIN( VMIN, V(I) )
00026330          VMAX = MAX( VMAX, V(I) )
00026340
00026350      10 CONTINUE
00026360
00026370      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00026380      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00026390      VINC = (VMAX-VMIN) / MG
00026400      I = 1
00026410      10 CONTINUE
00026420      IF ( A .LT. 0.0 ) I = 1 - I
00026430      SCALE = 10.0 ** ( MD - I )
00026440      VINC = VINC * SCALE
00026450
00026460      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00026470      DO 30 I = 1, N
00026480          IF ( A GE VINC ) GO TO 50
00026490          30 CONTINUE
00026500      I = 1
00026510      40 CONTINUE
00026520      SCALE = SCALE / 10.0
00026530      50 CONTINUE
00026540      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00026550      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00026560      VINC = A / SCALE
00026570      A = VMIN / VINC
00026580      IF ( A .LT. 0.0 ) J = J - 1
00026590      SHIN = J * VINC
00026600      SMAX = SHIN + VINC = MG
00026610      IF ( SMAX GE VMAX ) GO TO 60
00026620      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00026630      IF ( I GE 99 ) GO TO 40
00026640      I = I + 1
00026650      60 RETURN
00026660      END
00026670
00026680      SUBROUTINE PLT20D ( A, V, SHAR, INHIN, VMAX, VMIN )
00026690
00026700      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00026710      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00026720      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00026730
00026740      A = ARRAY OF X COORDINATES
00026750      MPPL = NUMBER OF POINTS IN X AND Y
00026760      VMIN = MINIMUM X VALUE
00026770      VMAX = MAXIMUM X VALUE
00026780      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00026790      AND MAXIMUM VALUES
00026800      VMIN = MINIMUM Y VALUE
00026810      VMAX = MAXIMUM Y VALUE
00026820      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00026830      AND MAXIMUM VALUES
00026840
00026850      COMMON / TEMPT / IPSW
00026860      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00026870      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00026880      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00026890      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00026900      IF ( IPSW GT 0 ) PRINT 9001
00026910      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20D FROM SUBROUTINE PLOT *****' )
00026920      LINE = 5
00026930
00026940      IF ( A EQ 0 ) GO TO 20
00026950      VMIN = V(1)
00026960      VMAX = V(1)
00026970      DO 10 I = 2, MPPL
00026980          VMIN = MIN( VMIN, V(I) )
00026990          VMAX = MAX( VMAX, V(I) )
00027000
00027010      10 CONTINUE
00027020
00027030      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00027040      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00027050      VINC = (VMAX-VMIN) / MG
00027060      I = 1
00027070      10 CONTINUE
00027080      IF ( A .LT. 0.0 ) I = 1 - I
00027090      SCALE = 10.0 ** ( MD - I )
00027100      VINC = VINC * SCALE
00027110
00027120      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00027130      DO 30 I = 1, N
00027140          IF ( A GE VINC ) GO TO 50
00027150          30 CONTINUE
00027160      I = 1
00027170      40 CONTINUE
00027180      SCALE = SCALE / 10.0
00027190      50 CONTINUE
00027200      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00027210      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00027220      VINC = A / SCALE
00027230      A = VMIN / VINC
00027240      IF ( A .LT. 0.0 ) J = J - 1
00027250      SHIN = J * VINC
00027260      SMAX = SHIN + VINC = MG
00027270      IF ( SMAX GE VMAX ) GO TO 60
00027280      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00027290      IF ( I GE 99 ) GO TO 40
00027300      I = I + 1
00027310      60 RETURN
00027320      END
00027330
00027340      SUBROUTINE PLT20E ( A, V, SHAR, INHIN, VMAX, VMIN )
00027350
00027360      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00027370      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00027380      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00027390
00027400      A = ARRAY OF X COORDINATES
00027410      MPPL = NUMBER OF POINTS IN X AND Y
00027420      VMIN = MINIMUM X VALUE
00027430      VMAX = MAXIMUM X VALUE
00027440      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00027450      AND MAXIMUM VALUES
00027460      VMIN = MINIMUM Y VALUE
00027470      VMAX = MAXIMUM Y VALUE
00027480      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00027490      AND MAXIMUM VALUES
00027500
00027510      COMMON / TEMPT / IPSW
00027520      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00027530      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00027540      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00027550      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00027560      IF ( IPSW GT 0 ) PRINT 9001
00027570      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20E FROM SUBROUTINE PLOT *****' )
00027580      LINE = 5
00027590
00027600      IF ( A EQ 0 ) GO TO 20
00027610      VMIN = V(1)
00027620      VMAX = V(1)
00027630      DO 10 I = 2, MPPL
00027640          VMIN = MIN( VMIN, V(I) )
00027650          VMAX = MAX( VMAX, V(I) )
00027660
00027670      10 CONTINUE
00027680
00027690      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00027700      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00027710      VINC = (VMAX-VMIN) / MG
00027720      I = 1
00027730      10 CONTINUE
00027740      IF ( A .LT. 0.0 ) I = 1 - I
00027750      SCALE = 10.0 ** ( MD - I )
00027760      VINC = VINC * SCALE
00027770
00027780      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00027790      DO 30 I = 1, N
00027800          IF ( A GE VINC ) GO TO 50
00027810          30 CONTINUE
00027820      I = 1
00027830      40 CONTINUE
00027840      SCALE = SCALE / 10.0
00027850      50 CONTINUE
00027860      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00027870      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00027880      VINC = A / SCALE
00027890      A = VMIN / VINC
00027900      IF ( A .LT. 0.0 ) J = J - 1
00027910      SHIN = J * VINC
00027920      SMAX = SHIN + VINC = MG
00027930      IF ( SMAX GE VMAX ) GO TO 60
00027940      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00027950      IF ( I GE 99 ) GO TO 40
00027960      I = I + 1
00027970      60 RETURN
00027980      END
00027990
00028000      SUBROUTINE PLT20F ( A, V, SHAR, INHIN, VMAX, VMIN )
00028010
00028020      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00028030      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00028040      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00028050
00028060      A = ARRAY OF X COORDINATES
00028070      MPPL = NUMBER OF POINTS IN X AND Y
00028080      VMIN = MINIMUM X VALUE
00028090      VMAX = MAXIMUM X VALUE
00028100      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00028110      AND MAXIMUM VALUES
00028120      VMIN = MINIMUM Y VALUE
00028130      VMAX = MAXIMUM Y VALUE
00028140      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00028150      AND MAXIMUM VALUES
00028160
00028170      COMMON / TEMPT / IPSW
00028180      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00028190      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00028200      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00028210      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00028220      IF ( IPSW GT 0 ) PRINT 9001
00028230      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20F FROM SUBROUTINE PLOT *****' )
00028240      LINE = 5
00028250
00028260      IF ( A EQ 0 ) GO TO 20
00028270      VMIN = V(1)
00028280      VMAX = V(1)
00028290      DO 10 I = 2, MPPL
00028300          VMIN = MIN( VMIN, V(I) )
00028310          VMAX = MAX( VMAX, V(I) )
00028320
00028330      10 CONTINUE
00028340
00028350      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00028360      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00028370      VINC = (VMAX-VMIN) / MG
00028380      I = 1
00028390      10 CONTINUE
00028400      IF ( A .LT. 0.0 ) I = 1 - I
00028410      SCALE = 10.0 ** ( MD - I )
00028420      VINC = VINC * SCALE
00028430
00028440      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00028450      DO 30 I = 1, N
00028460          IF ( A GE VINC ) GO TO 50
00028470          30 CONTINUE
00028480      I = 1
00028490      40 CONTINUE
00028500      SCALE = SCALE / 10.0
00028510      50 CONTINUE
00028520      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00028530      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00028540      VINC = A / SCALE
00028550      A = VMIN / VINC
00028560      IF ( A .LT. 0.0 ) J = J - 1
00028570      SHIN = J * VINC
00028580      SMAX = SHIN + VINC = MG
00028590      IF ( SMAX GE VMAX ) GO TO 60
00028600      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00028610      IF ( I GE 99 ) GO TO 40
00028620      I = I + 1
00028630      60 RETURN
00028640      END
00028650
00028660      SUBROUTINE PLT20G ( A, V, SHAR, INHIN, VMAX, VMIN )
00028670
00028680      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00028690      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00028700      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00028710
00028720      A = ARRAY OF X COORDINATES
00028730      MPPL = NUMBER OF POINTS IN X AND Y
00028740      VMIN = MINIMUM X VALUE
00028750      VMAX = MAXIMUM X VALUE
00028760      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00028770      AND MAXIMUM VALUES
00028780      VMIN = MINIMUM Y VALUE
00028790      VMAX = MAXIMUM Y VALUE
00028800      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00028810      AND MAXIMUM VALUES
00028820
00028830      COMMON / TEMPT / IPSW
00028840      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00028850      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00028860      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00028870      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00028880      IF ( IPSW GT 0 ) PRINT 9001
00028890      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20G FROM SUBROUTINE PLOT *****' )
00028900      LINE = 5
00028910
00028920      IF ( A EQ 0 ) GO TO 20
00028930      VMIN = V(1)
00028940      VMAX = V(1)
00028950      DO 10 I = 2, MPPL
00028960          VMIN = MIN( VMIN, V(I) )
00028970          VMAX = MAX( VMAX, V(I) )
00028980
00028990      10 CONTINUE
00029000
00029010      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00029020      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00029030      VINC = (VMAX-VMIN) / MG
00029040      I = 1
00029050      10 CONTINUE
00029060      IF ( A .LT. 0.0 ) I = 1 - I
00029070      SCALE = 10.0 ** ( MD - I )
00029080      VINC = VINC * SCALE
00029090
00029100      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00029110      DO 30 I = 1, N
00029120          IF ( A GE VINC ) GO TO 50
00029130          30 CONTINUE
00029140      I = 1
00029150      40 CONTINUE
00029160      SCALE = SCALE / 10.0
00029170      50 CONTINUE
00029180      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00029190      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00029200      VINC = A / SCALE
00029210      A = VMIN / VINC
00029220      IF ( A .LT. 0.0 ) J = J - 1
00029230      SHIN = J * VINC
00029240      SMAX = SHIN + VINC = MG
00029250      IF ( SMAX GE VMAX ) GO TO 60
00029260      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00029270      IF ( I GE 99 ) GO TO 40
00029280      I = I + 1
00029290      60 RETURN
00029300      END
00029310
00029320      SUBROUTINE PLT20H ( A, V, SHAR, INHIN, VMAX, VMIN )
00029330
00029340      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00029350      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00029360      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00029370
00029380      A = ARRAY OF X COORDINATES
00029390      MPPL = NUMBER OF POINTS IN X AND Y
00029400      VMIN = MINIMUM X VALUE
00029410      VMAX = MAXIMUM X VALUE
00029420      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00029430      AND MAXIMUM VALUES
00029440      VMIN = MINIMUM Y VALUE
00029450      VMAX = MAXIMUM Y VALUE
00029460      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00029470      AND MAXIMUM VALUES
00029480
00029490      COMMON / TEMPT / IPSW
00029500      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00029510      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00029520      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00029530      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00029540      IF ( IPSW GT 0 ) PRINT 9001
00029550      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20H FROM SUBROUTINE PLOT *****' )
00029560      LINE = 5
00029570
00029580      IF ( A EQ 0 ) GO TO 20
00029590      VMIN = V(1)
00029600      VMAX = V(1)
00029610      DO 10 I = 2, MPPL
00029620          VMIN = MIN( VMIN, V(I) )
00029630          VMAX = MAX( VMAX, V(I) )
00029640
00029650      10 CONTINUE
00029660
00029670      SCALE (VMAX-VMIN)/MG TO HAVE SAME NO. DIGITS AS NO. ENTRIES IN ALLOW
00029680      MD = 1/FIX ( 1.01 * ALOG10 ( FLOAT( ALLOW(1) ) ) )
00029690      VINC = (VMAX-VMIN) / MG
00029700      I = 1
00029710      10 CONTINUE
00029720      IF ( A .LT. 0.0 ) I = 1 - I
00029730      SCALE = 10.0 ** ( MD - I )
00029740      VINC = VINC * SCALE
00029750
00029760      FIND AN ELEMENT IN ALLOW THAT IS GREATER THAN OR EQUAL TO VINC
00029770      DO 30 I = 1, N
00029780          IF ( A GE VINC ) GO TO 50
00029790          30 CONTINUE
00029800      I = 1
00029810      40 CONTINUE
00029820      SCALE = SCALE / 10.0
00029830      50 CONTINUE
00029840      THE SMALLEST ALLOWABLE INCREMENT IS NOW A / SCALE
00029850      NOW PICK SHIN AND SMAX SO ZERO WILL BE AN INCREMENT VALUE
00029860      VINC = A / SCALE
00029870      A = VMIN / VINC
00029880      IF ( A .LT. 0.0 ) J = J - 1
00029890      SHIN = J * VINC
00029900      SMAX = SHIN + VINC = MG
00029910      IF ( SMAX GE VMAX ) GO TO 60
00029920      VINC IS TOO SMALL TO FIT THE ADJUSTED RANGE. INCREASE IT
00029930      IF ( I GE 99 ) GO TO 40
00029940      I = I + 1
00029950      60 RETURN
00029960      END
00029970
00029980      SUBROUTINE PLT20I ( A, V, SHAR, INHIN, VMAX, VMIN )
00029990
00030000      SUBROUTINE TO PRODUCE A PRINTER PLOT OF X VS Y
00030010      X AND Y ARE ASSUMED NOT TO EXCEED 9999
00030020      INTEGERS BEFORE PLOTTING. GRID SIZE IS 10 X 10.
00030030
00030040      A = ARRAY OF X COORDINATES
00030050      MPPL = NUMBER OF POINTS IN X AND Y
00030060      VMIN = MINIMUM X VALUE
00030070      VMAX = MAXIMUM X VALUE
00030080      IF SHAR = SHIN, A WILL BE SEARCHED TO FIND THE MINIMUM
00030090      AND MAXIMUM VALUES
00030100      VMIN = MINIMUM Y VALUE
00030110      VMAX = MAXIMUM Y VALUE
00030120      IF VMAX = VMIN, Y WILL BE SEARCHED TO FIND THE MINIMUM
00030130      AND MAXIMUM VALUES
00030140
00030150      COMMON / TEMPT / IPSW
00030160      COMMON / PLOTS / MPPL, HEADPR(122), SURCAS(17)
00030170      DIMENSION I(1:10), ZK(1:1), X(1:1), Y(1:1)
00030180      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00030190      DATA I / 50, 10, 10, 10, 10, 10, 10, 10, 10, 10 /
00030200      IF ( IPSW GT 0 ) PRINT 9001
00030210      9001 FORMAT ( '***** ENTRY SUBROUTINE PLT20I FROM SUBROUT
```

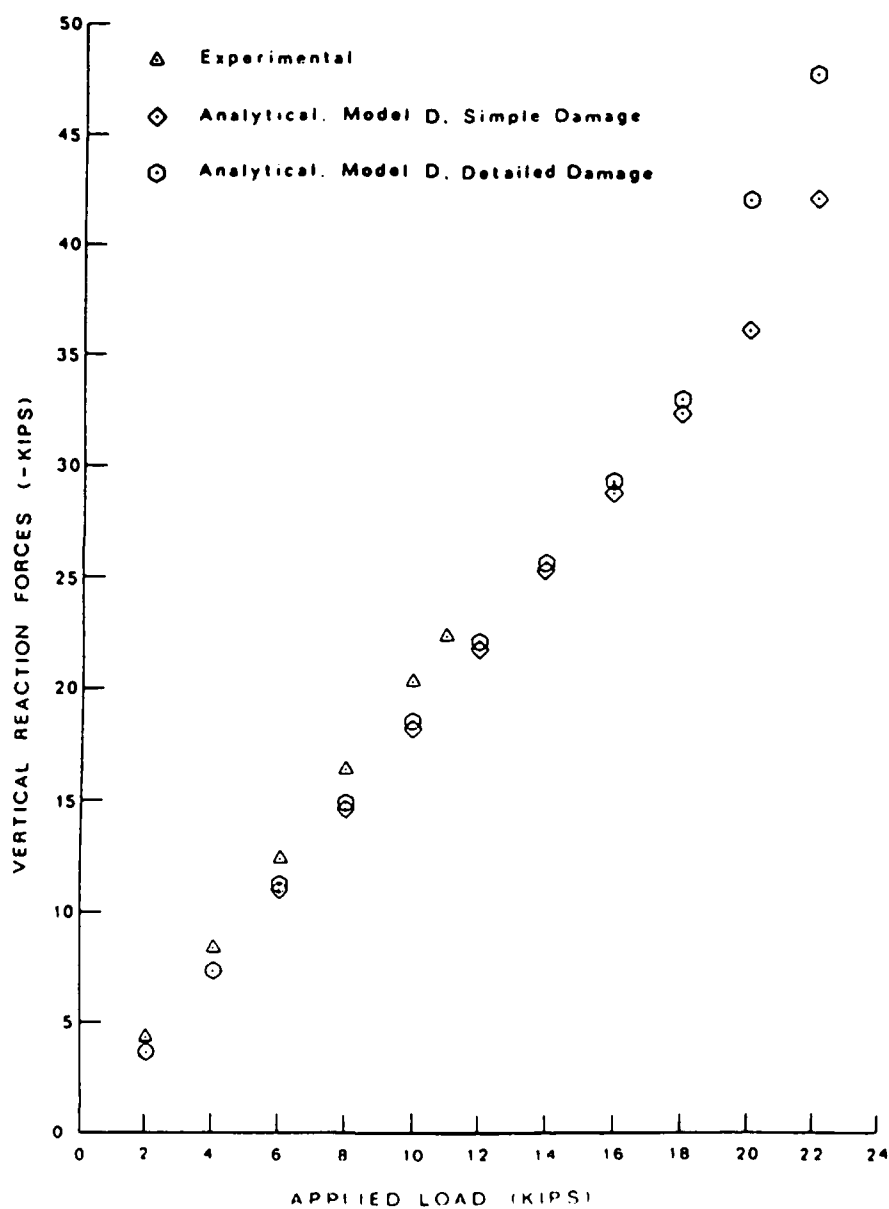
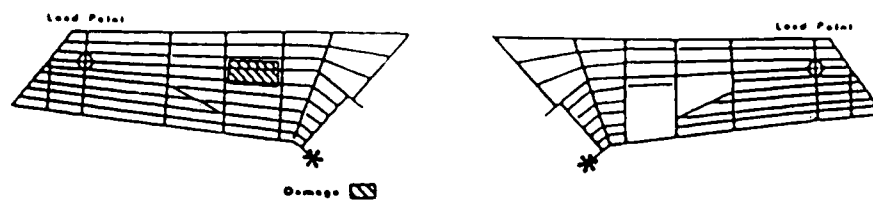
APPENDIX I

PLOTS OF ANALYTICAL AND EXPERIMENTAL DATA



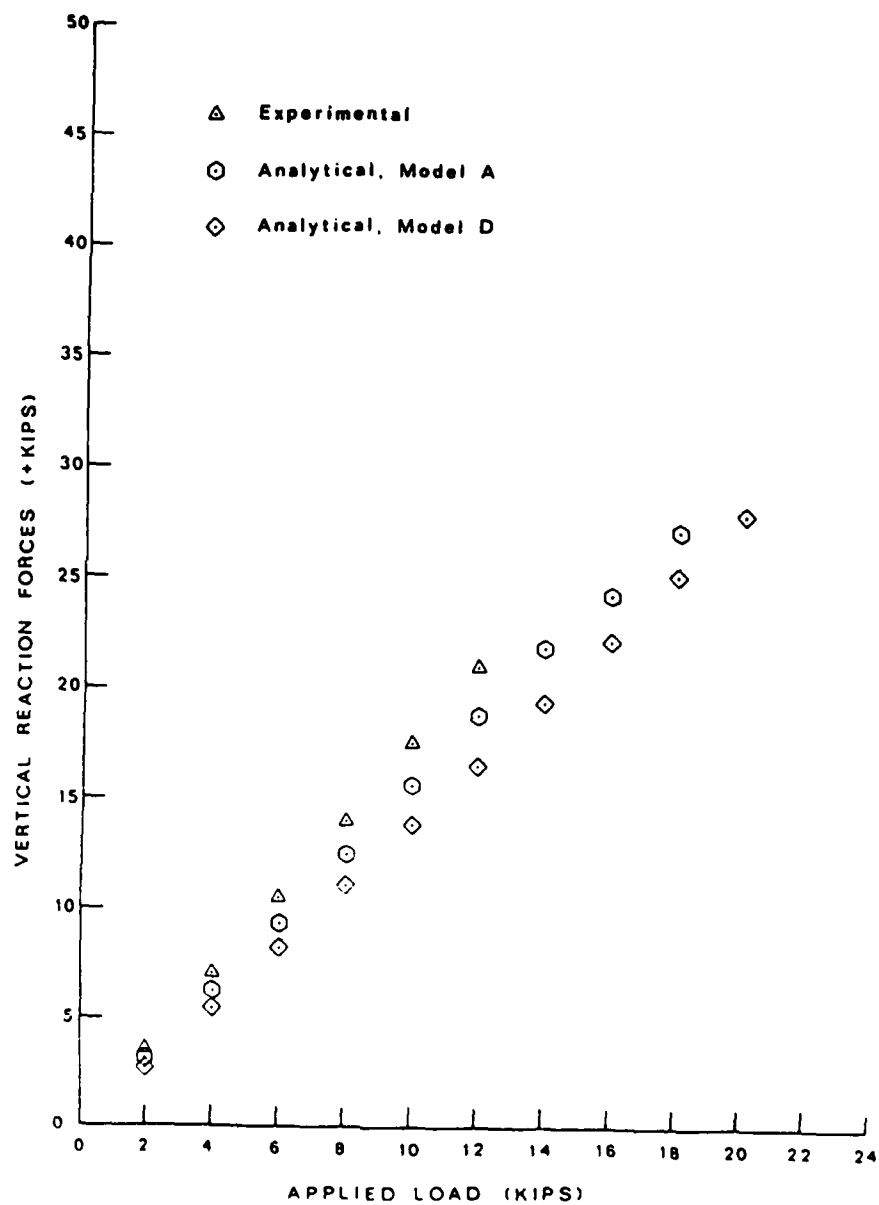
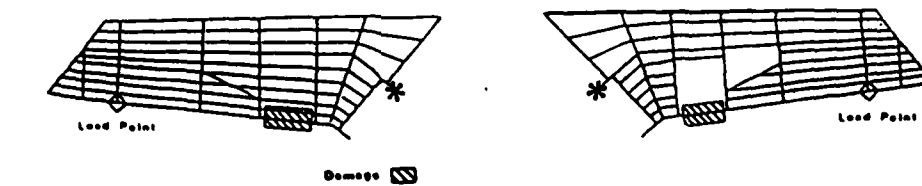
(a) Test 1, Front Spar

Figure 26. Comparison of Vertical Reaction Forces



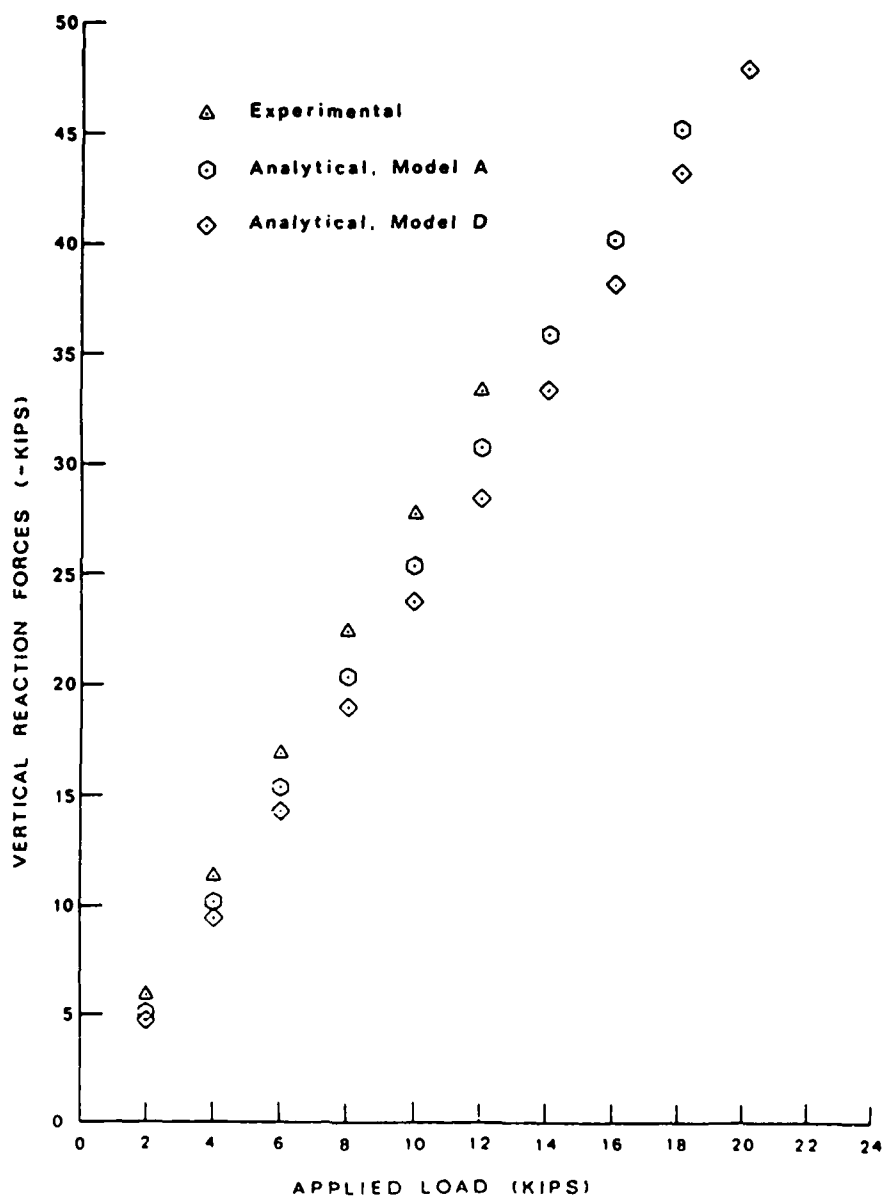
(b) Test I, Rear Spar

Figure 26. (Continued)



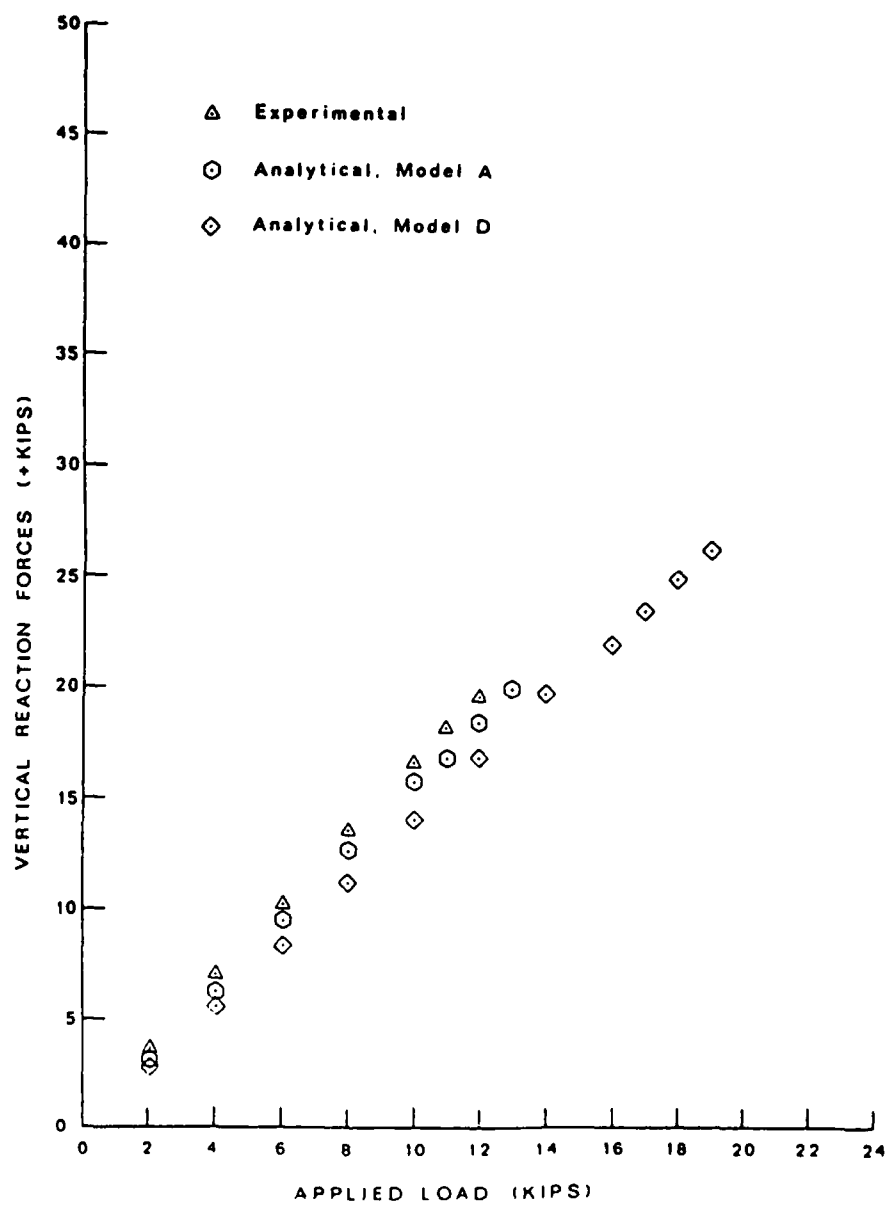
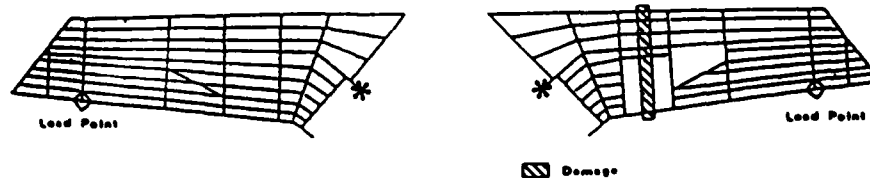
(C) Test 2C, Front Spar

Figure 26. (Continued)



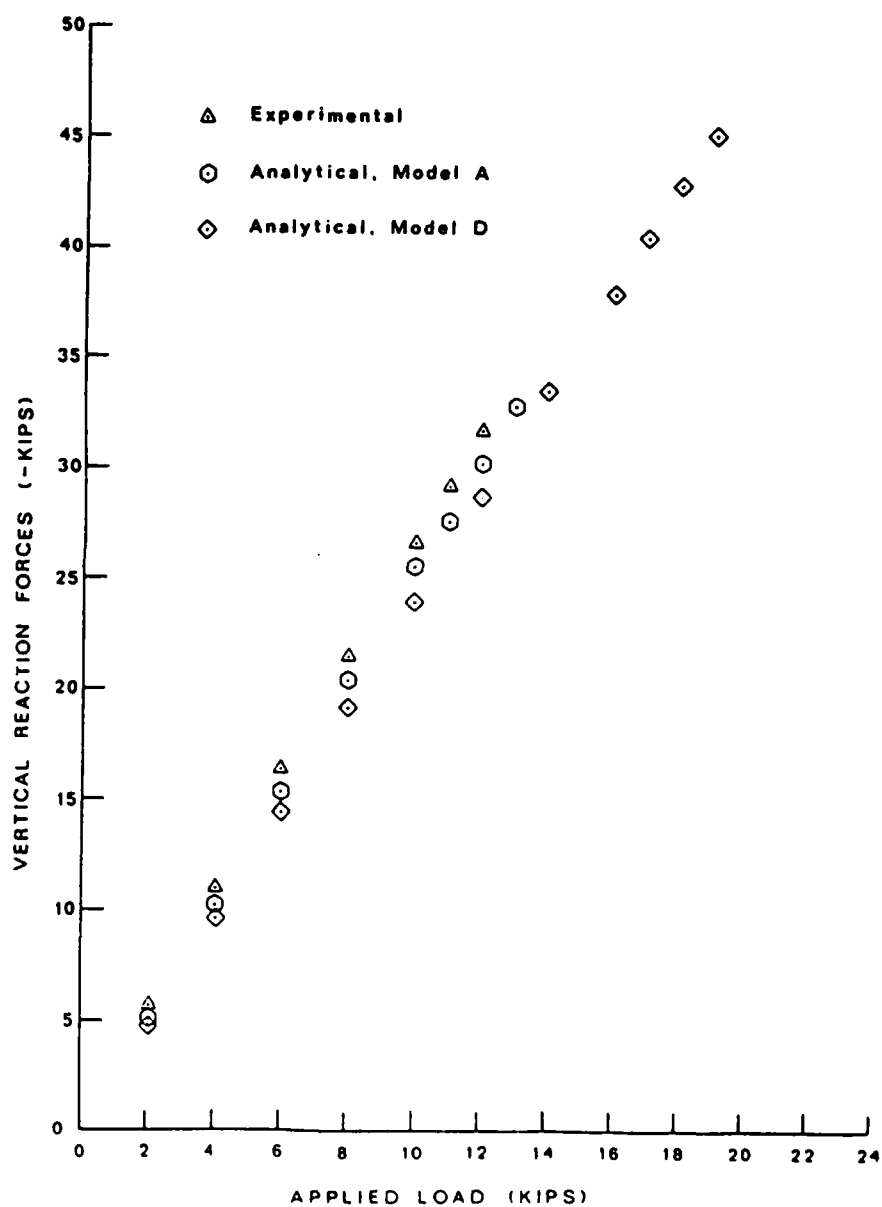
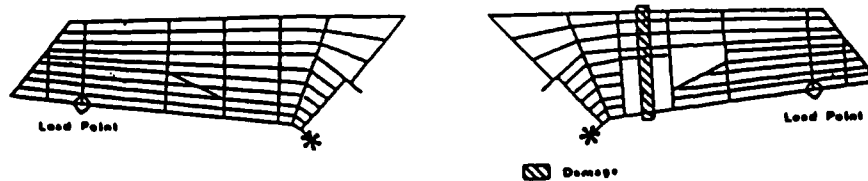
(d) Test 2C, Rear Spar

Figure 26. (Continued)



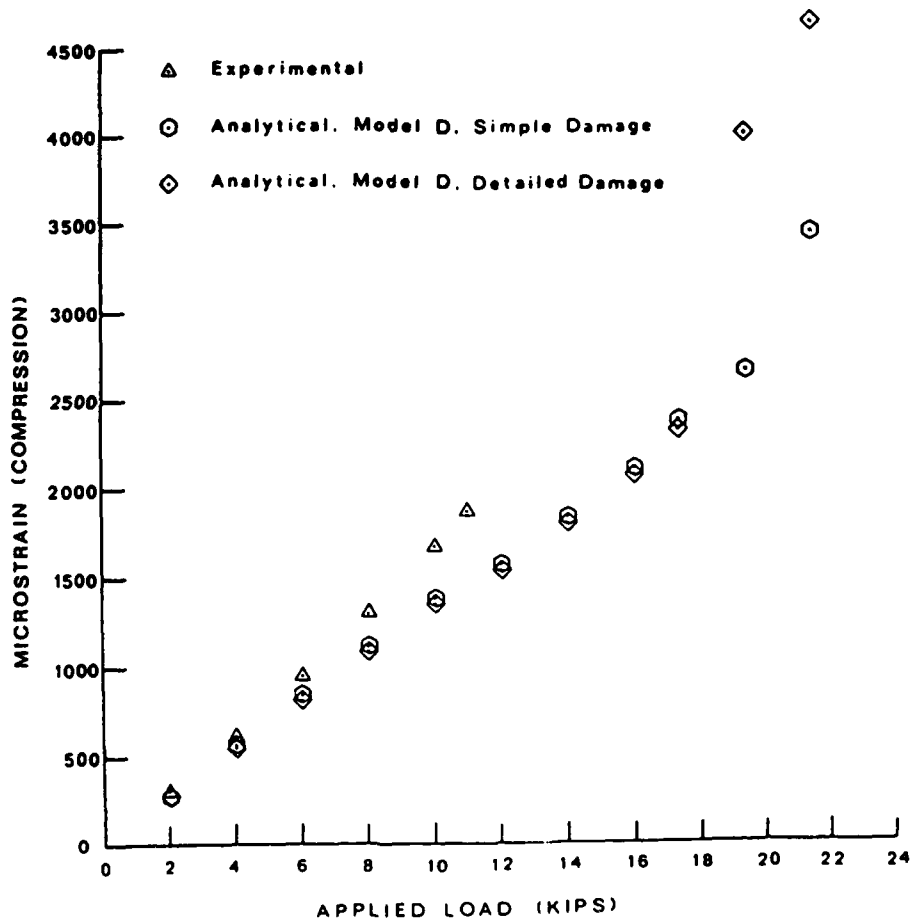
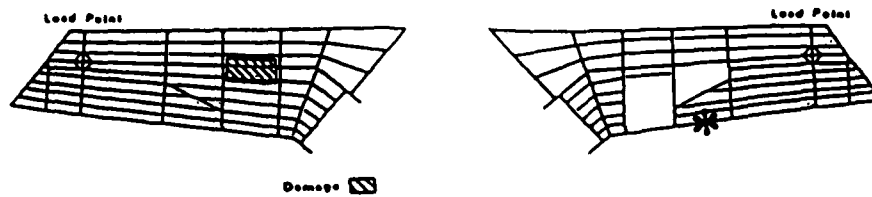
(e) Test 3B, Front Spar

Figure 26. (Continued)



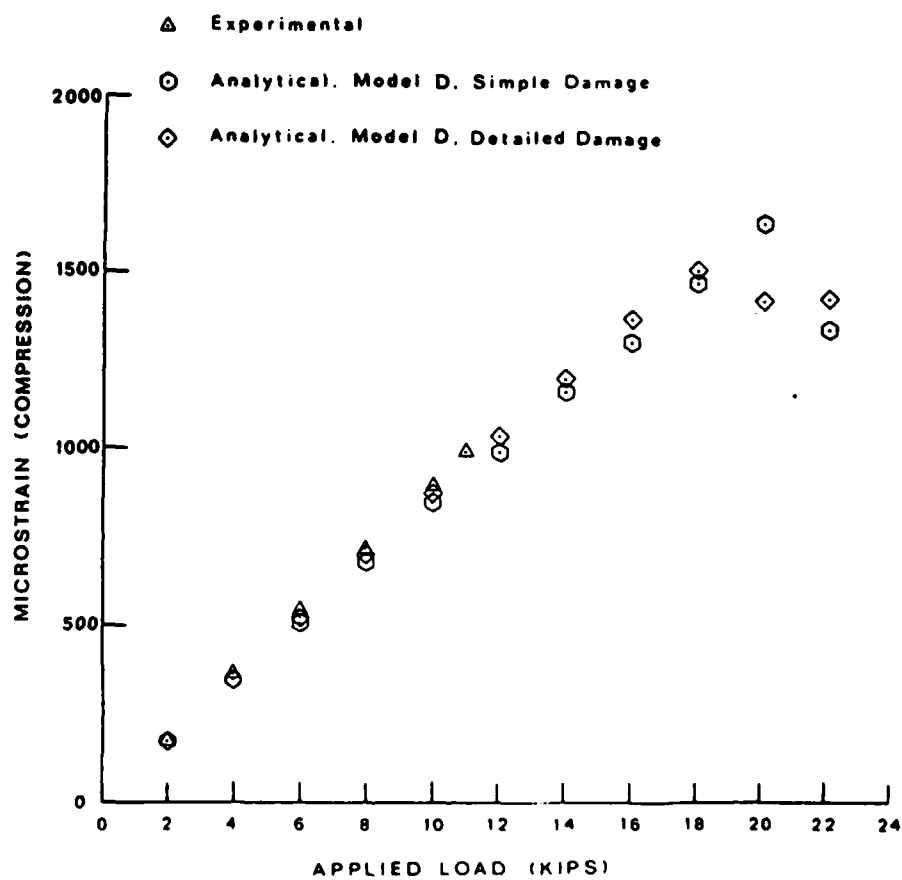
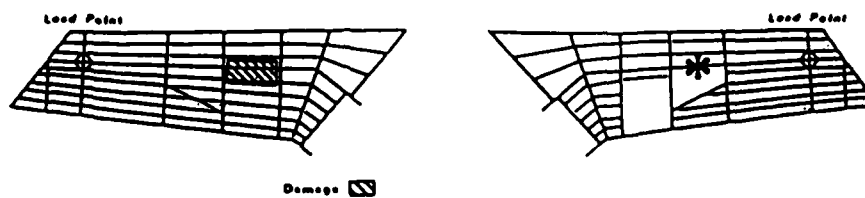
(f) Test 3B, Rear Spar

Figure 26. (Continued)



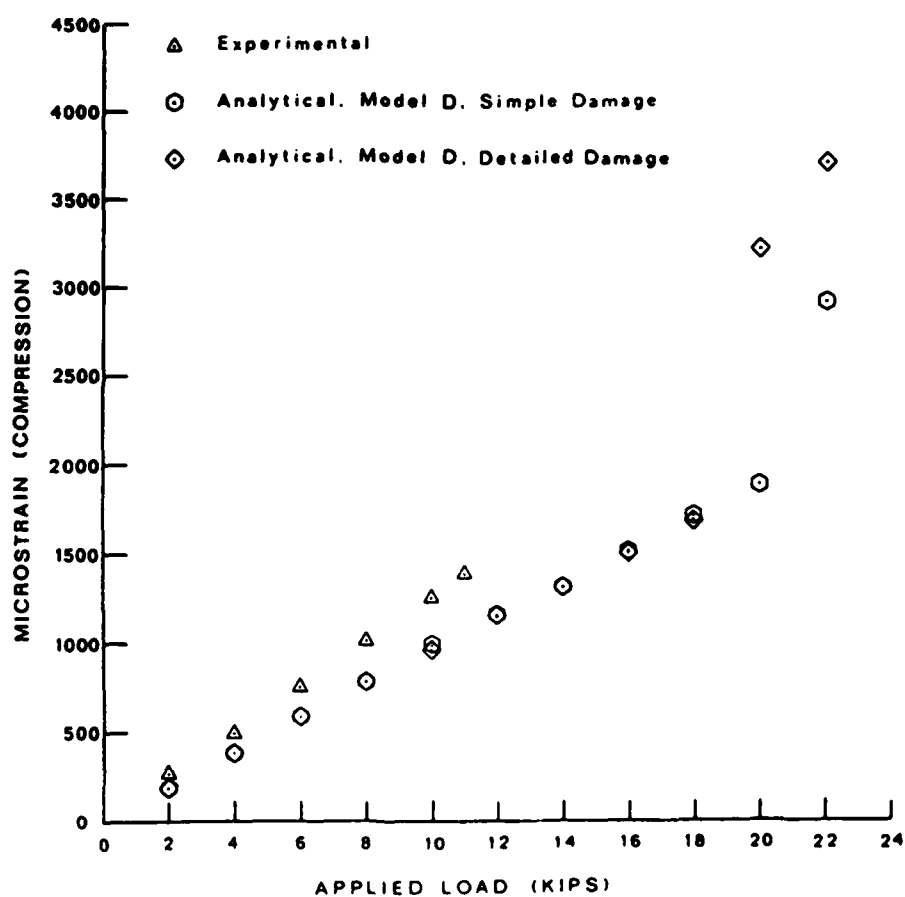
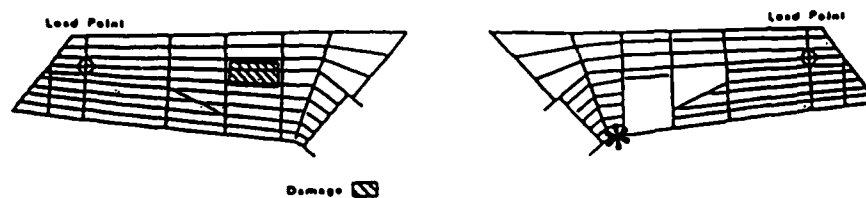
(a) Rod Element 458

Figure 27. Comparison of Strains for Test 1



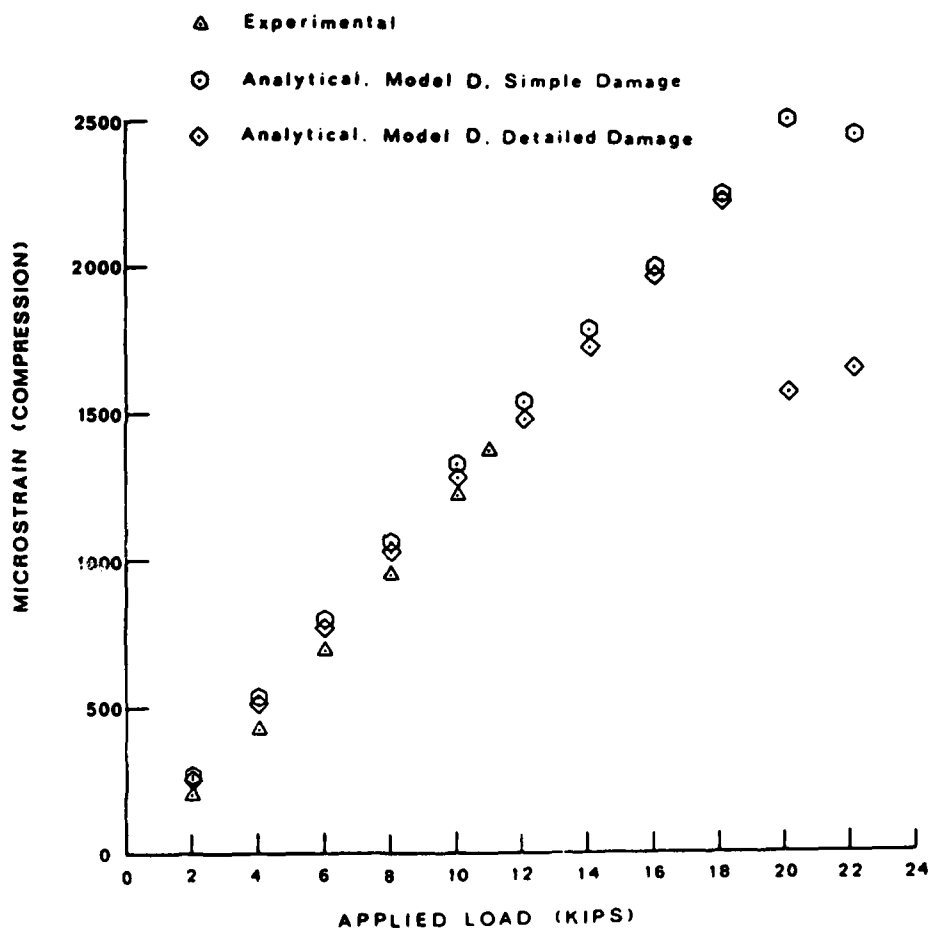
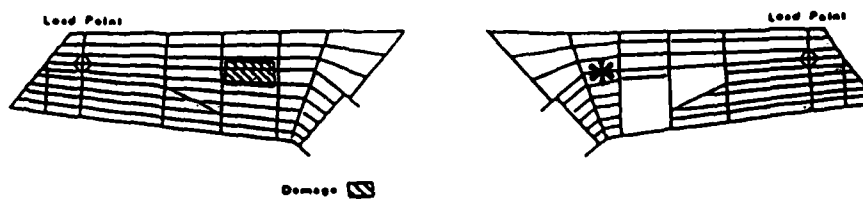
(b) Rod Element 476

Figure 27. (Continued)



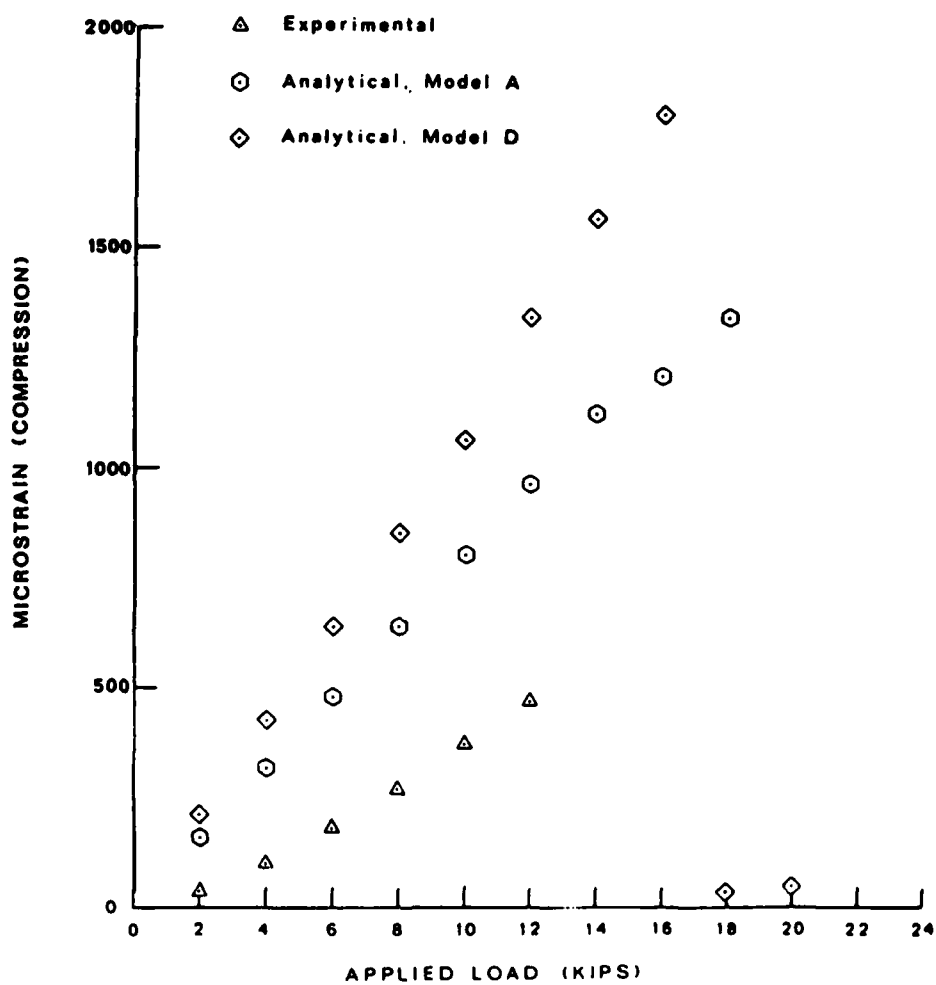
(C) Rod Element 564

Figure 27. (Continued)



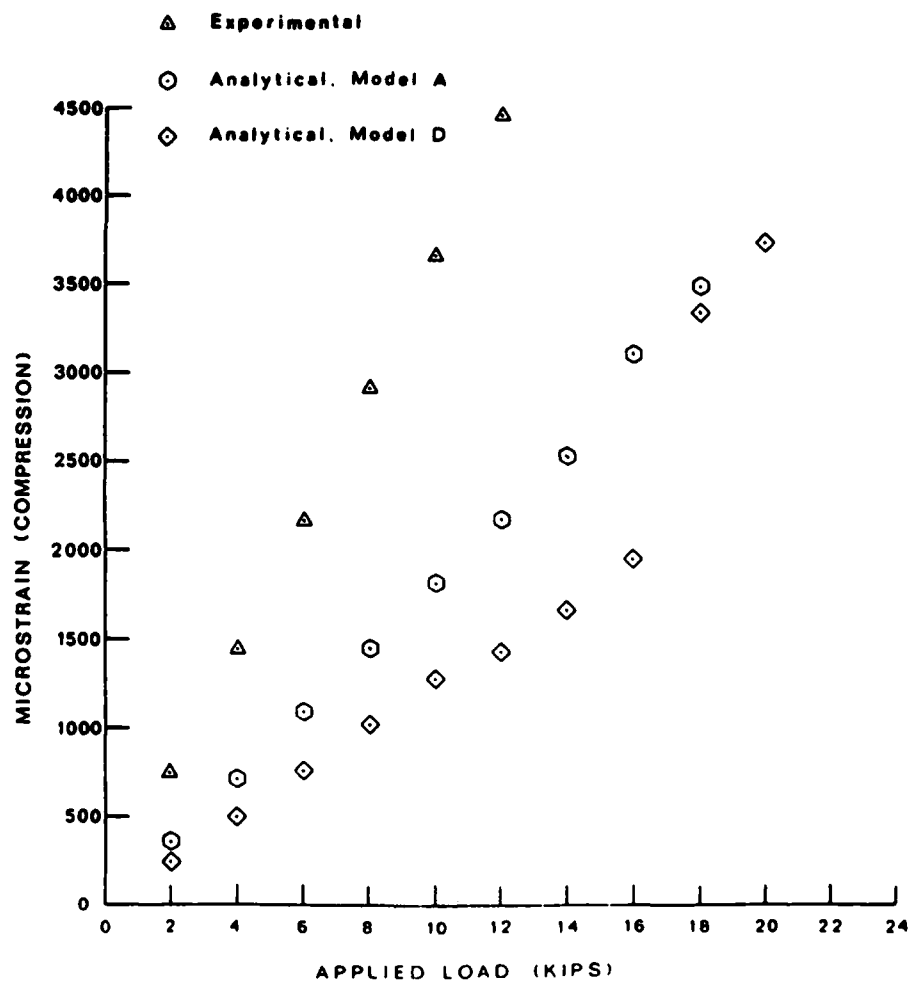
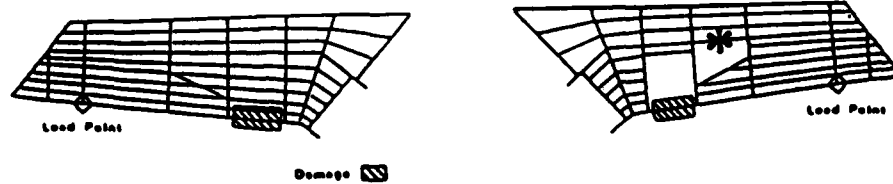
(d) Rod Element 576

Figure 27. (Continued)



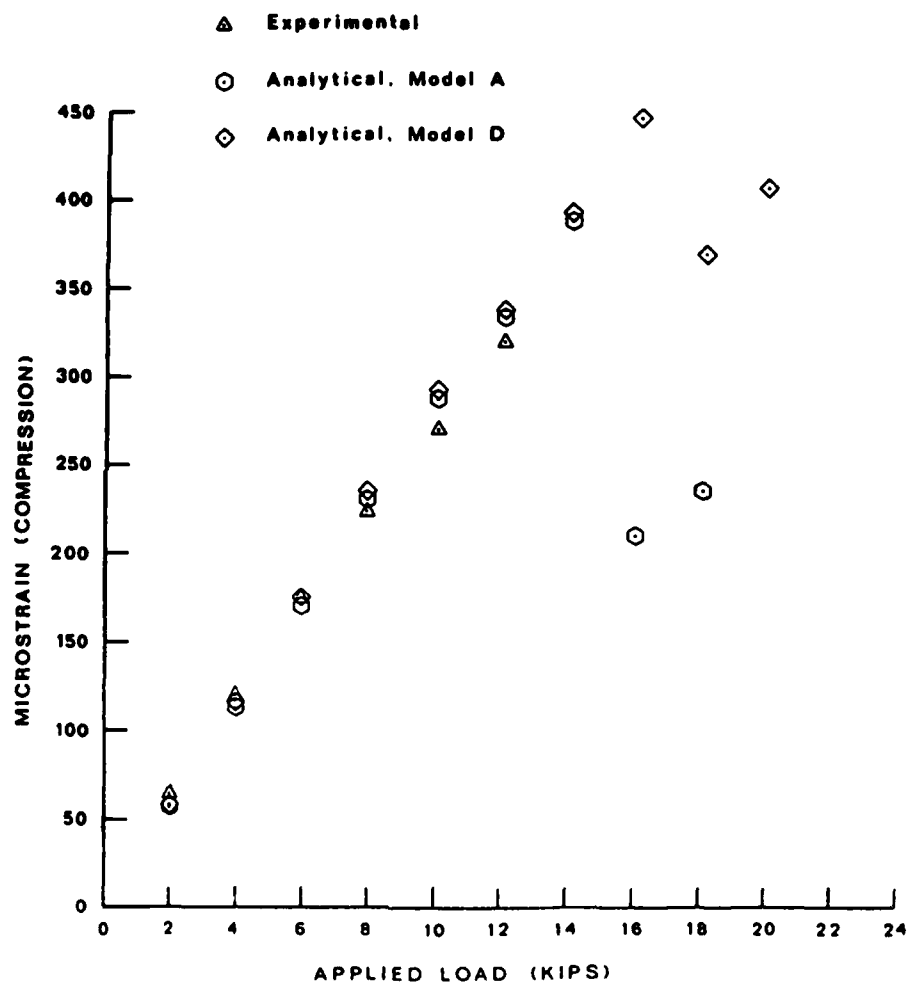
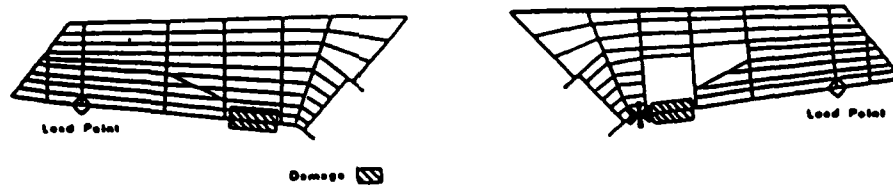
(a) Rod Element 458

Figure 28. Comparison of Strains for Test 2C



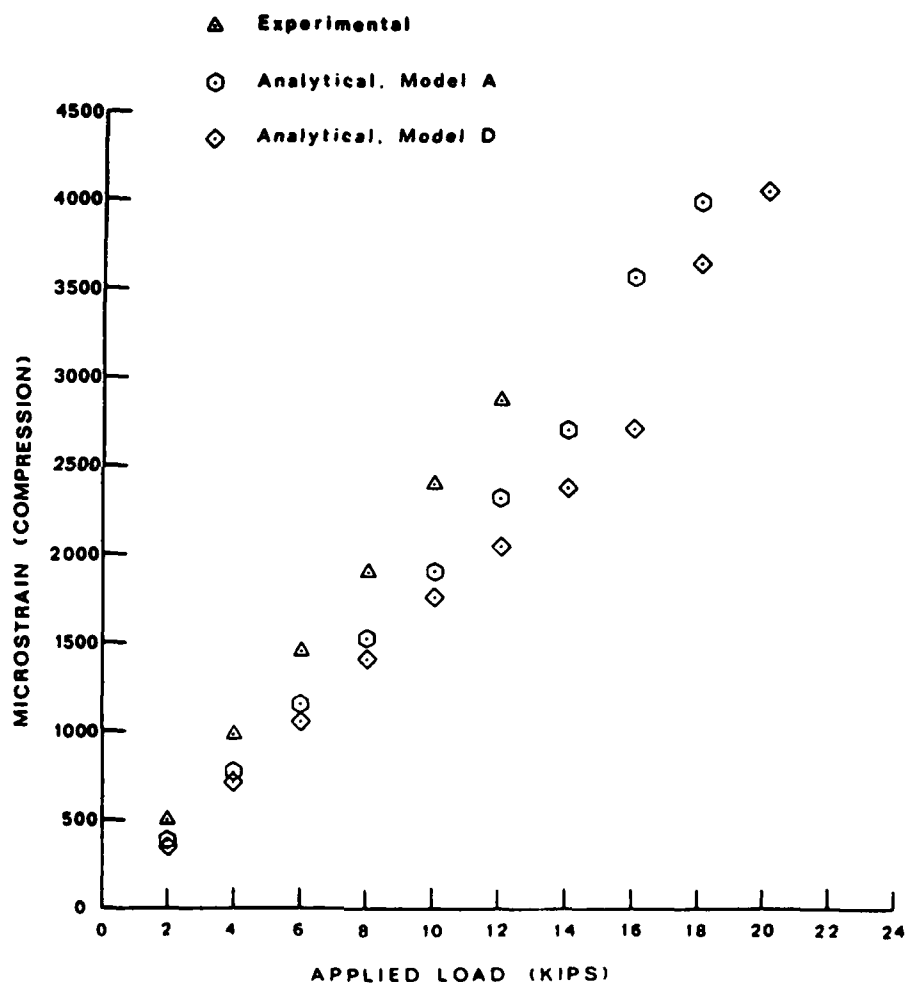
(b) Rod Element 476

Figure 28. (Continued)



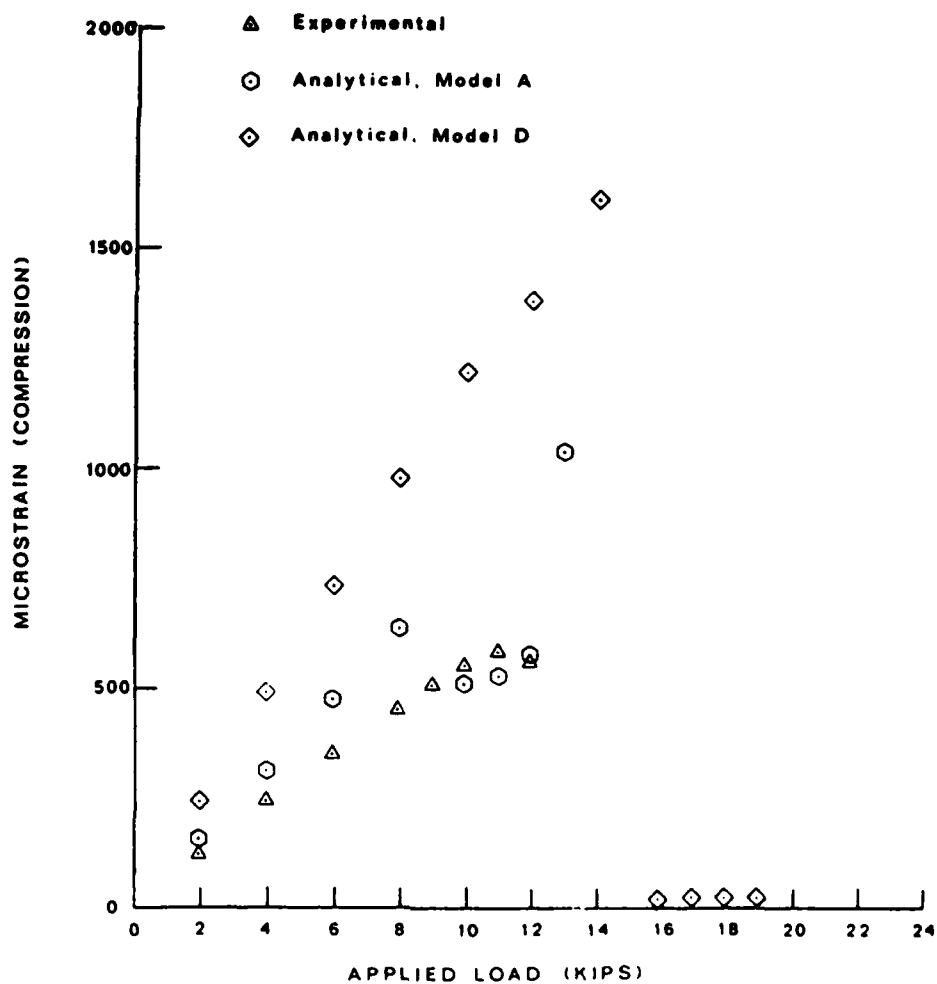
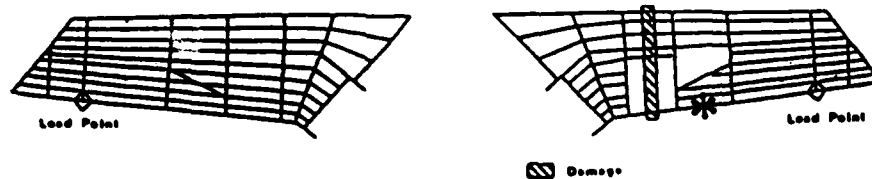
(C) Rod Element 564

Figure 28. (Continued)



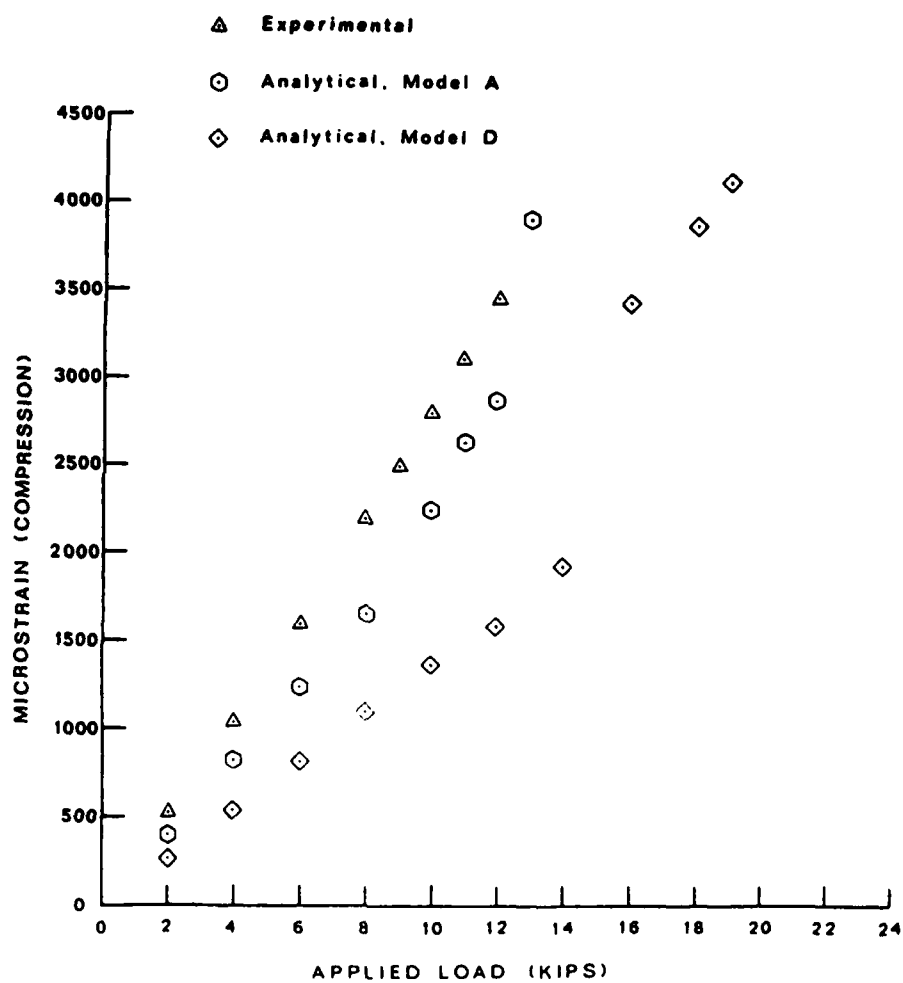
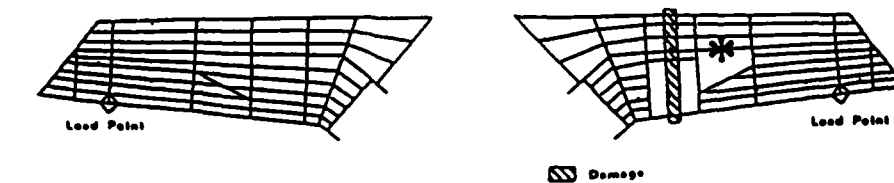
(d) Rod Element 576

Figure 28. (Continued)



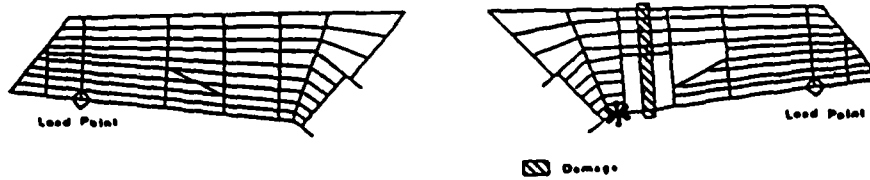
(■) Rod Element 458

Figure 29. Comparison of Strains for Test 38

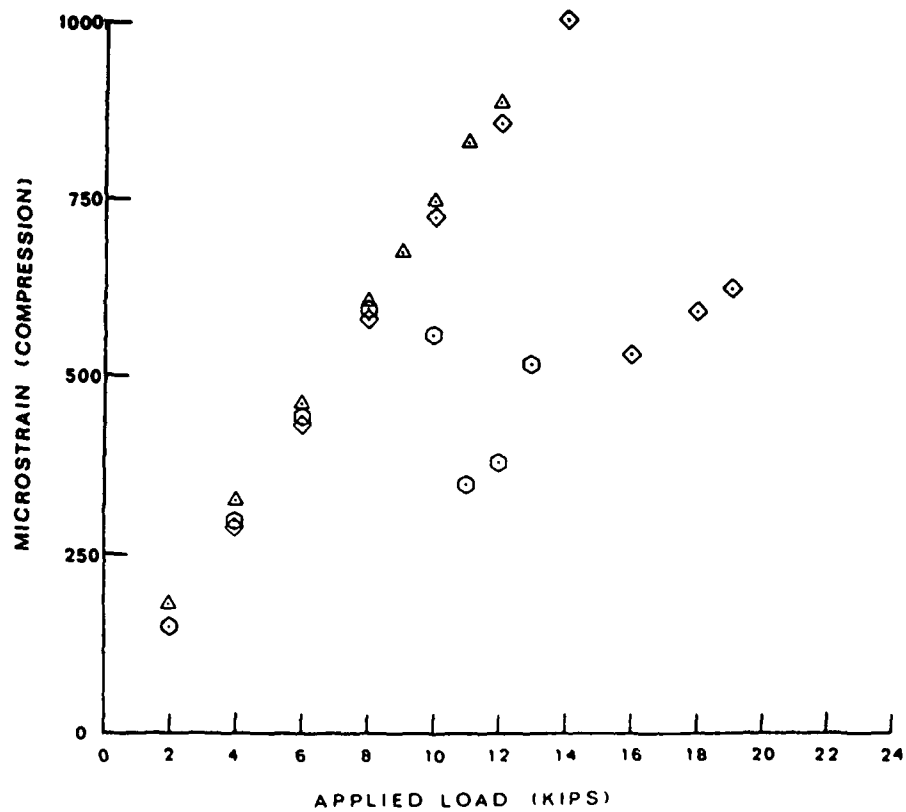


(b) Rod Element 476

Figure 29. (Continued)

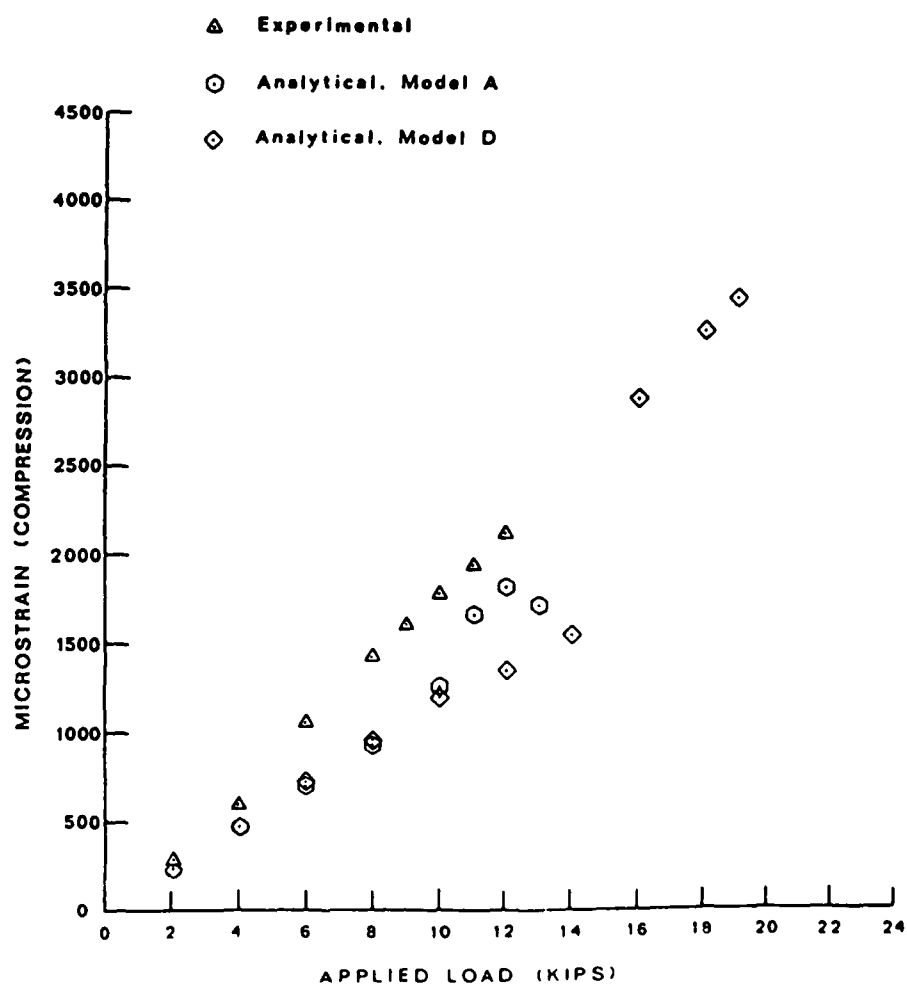


- △ Experimental
- Analytical, Model A
- ◇ Analytical, Model D



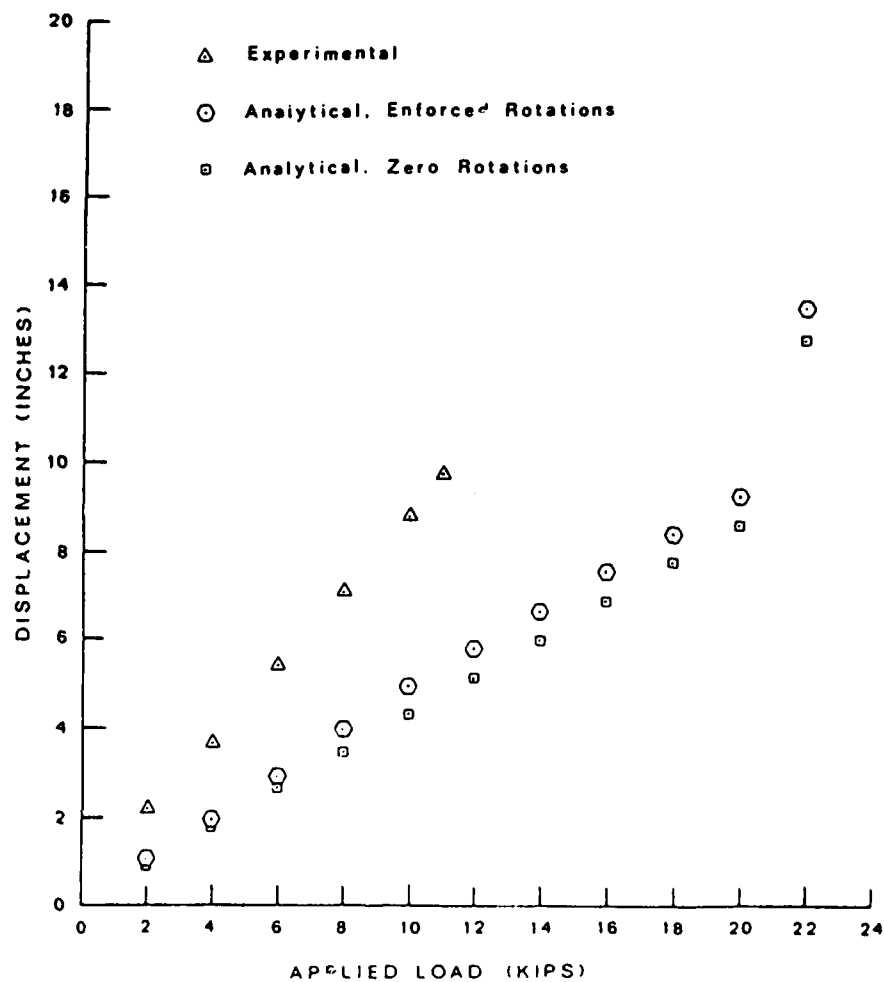
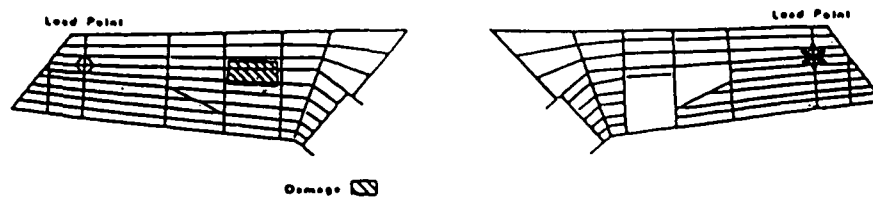
(C) Rod Element 564

Figure 29. (Continued)



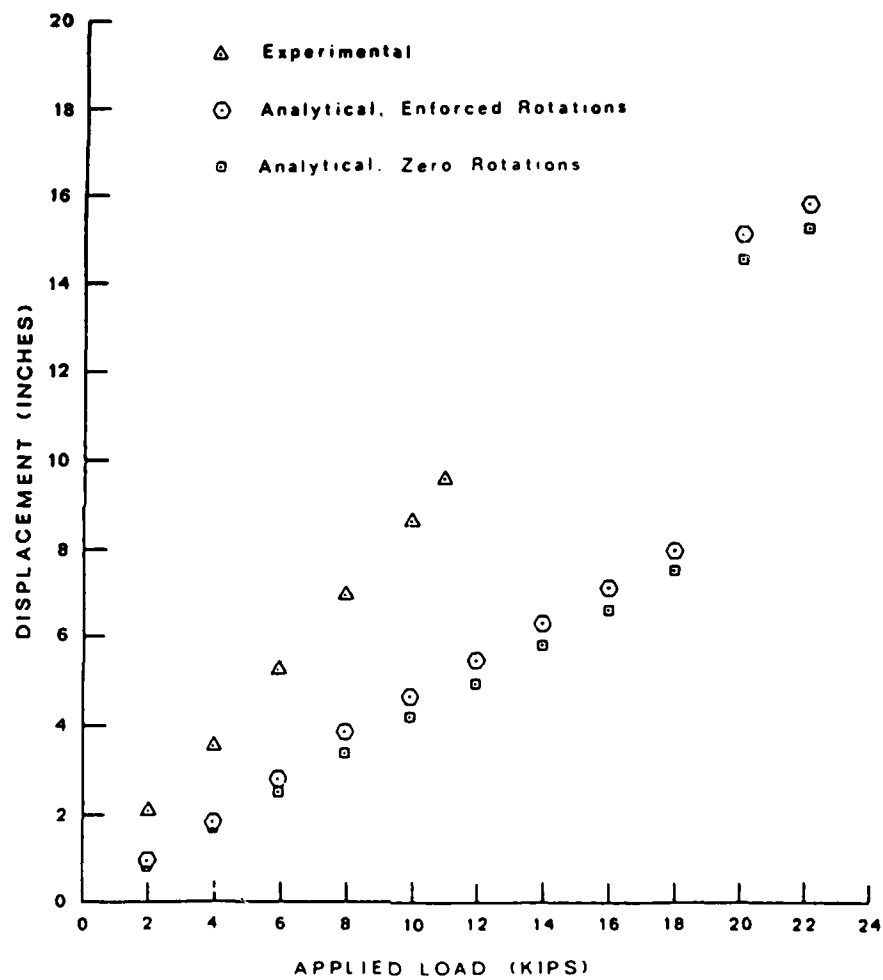
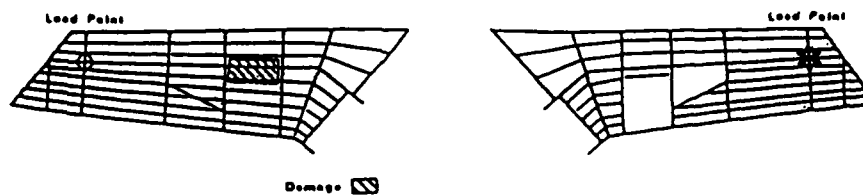
(d) Rod Element 576

Figure 29. (Continued)

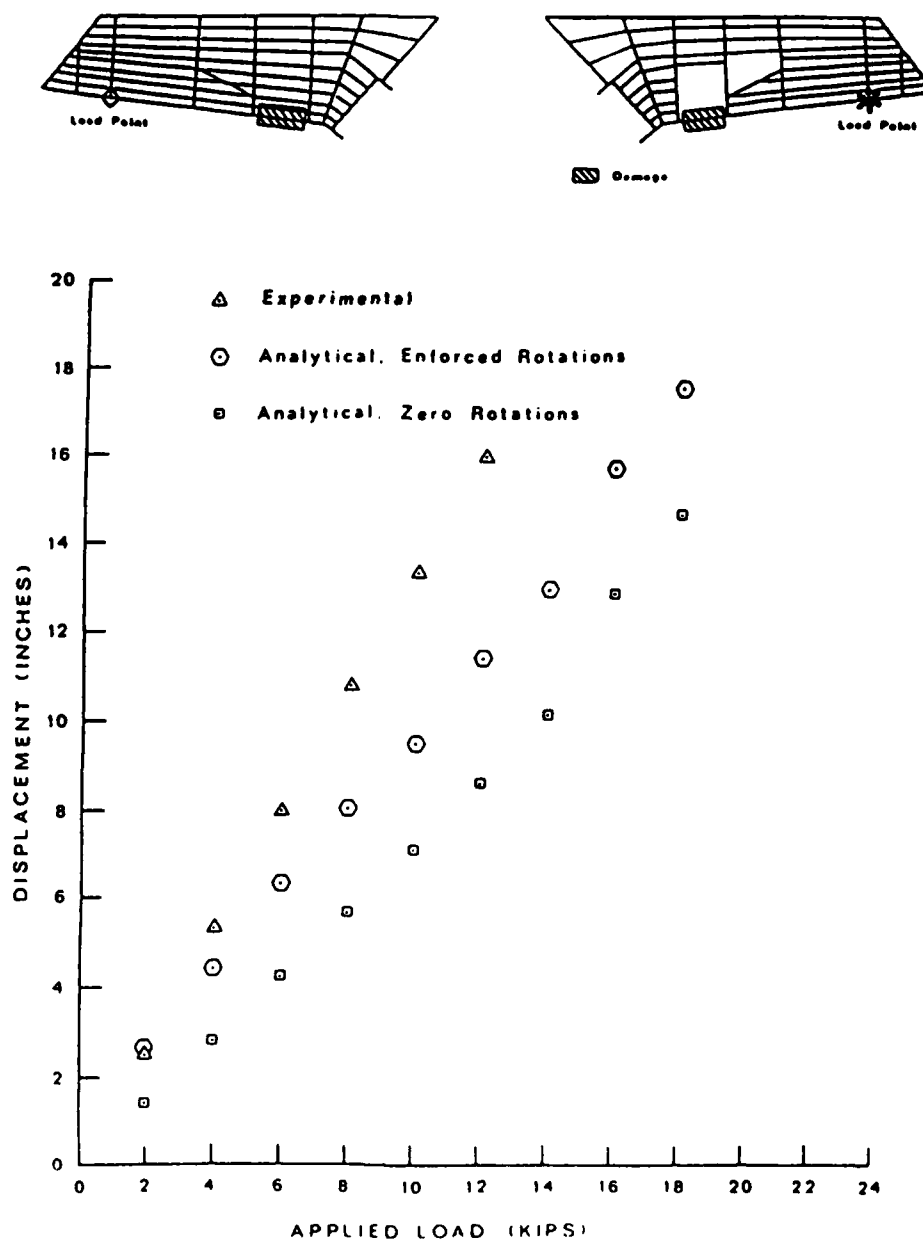


(a) Test 1, Model D (with torsional stiffness rods), Simple

Figure 30. Load Point Displacements for Test 1

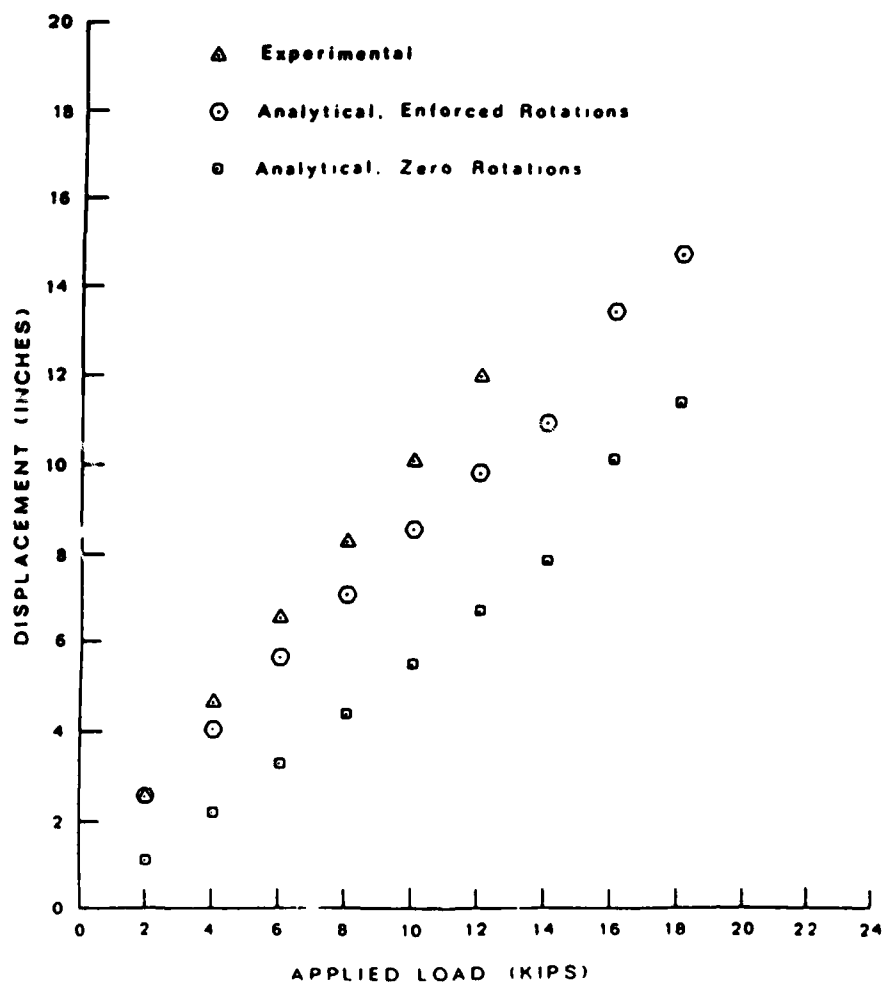


(b) Test 1, Model D (with torsional stiffness rods). Detailed
Figure 30. (Continued)



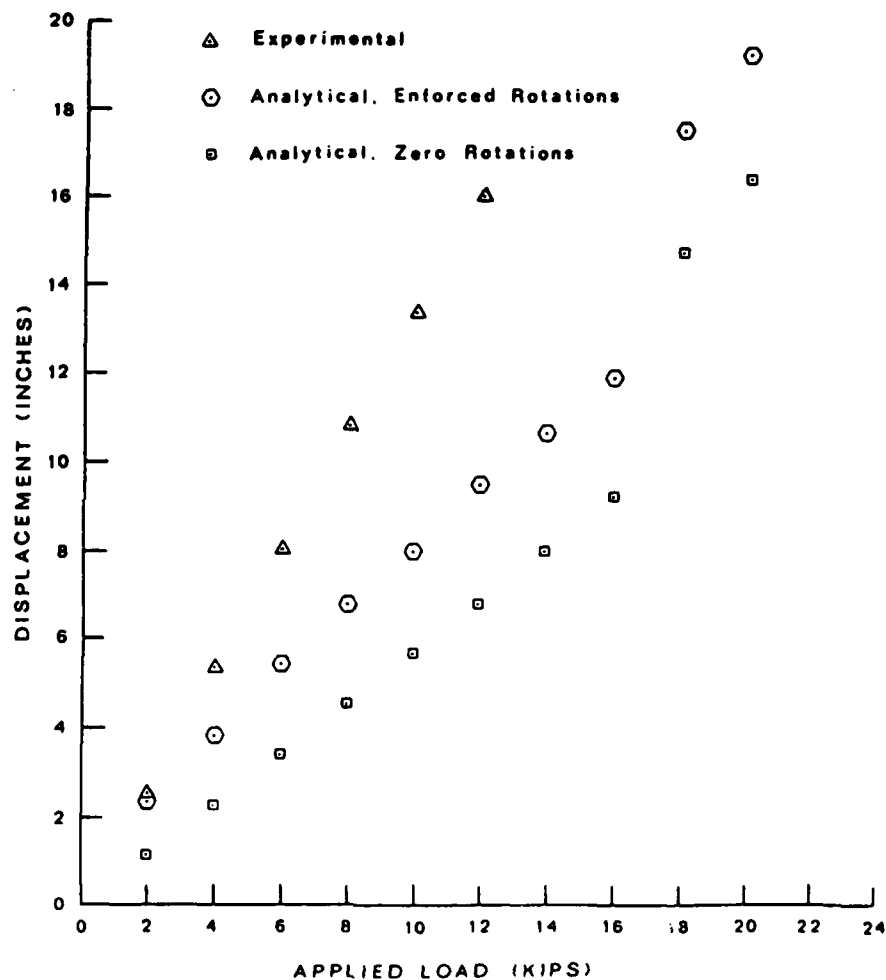
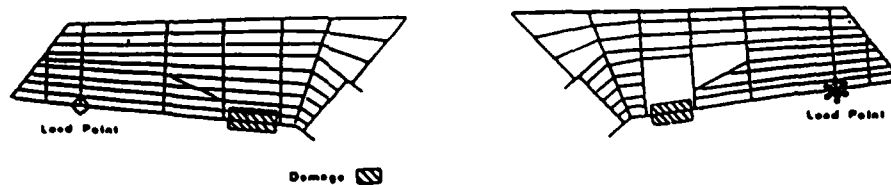
(a) Load Point, Model A (without torsional stiffness rods)

Figure 31. Single-Point Displacements for Test 2C



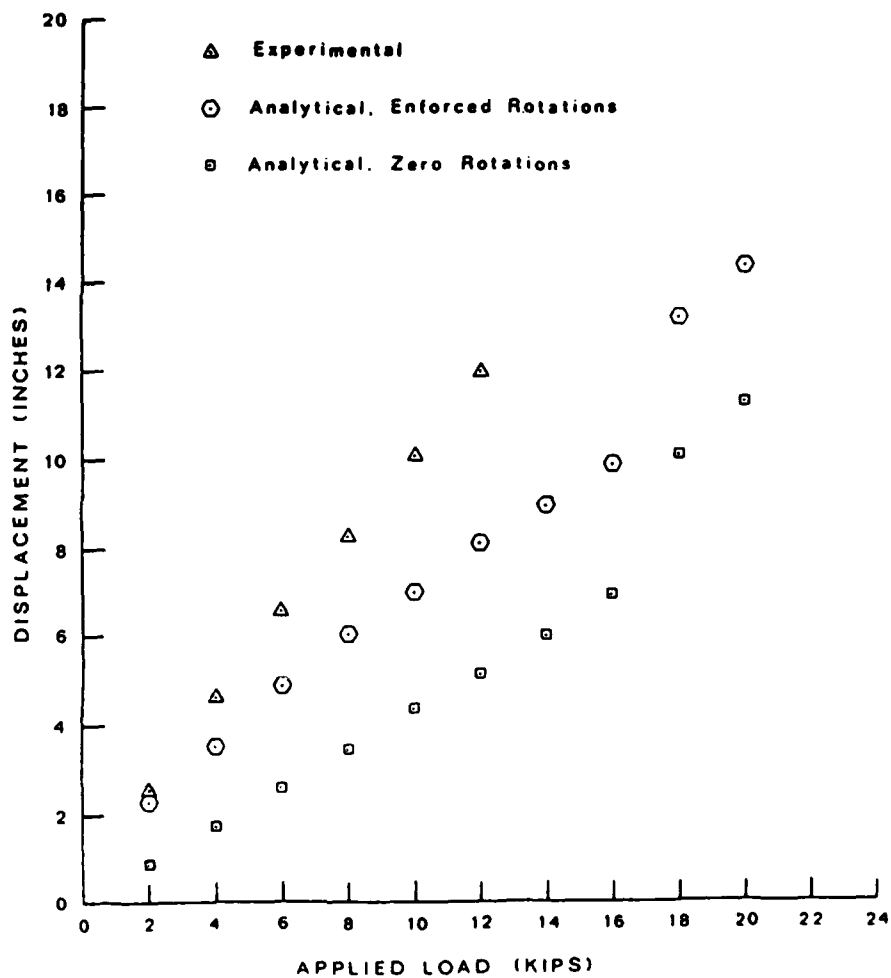
(b) Node 46, Model A (without torsional stiffness rods)

Figure 31. (Continued)



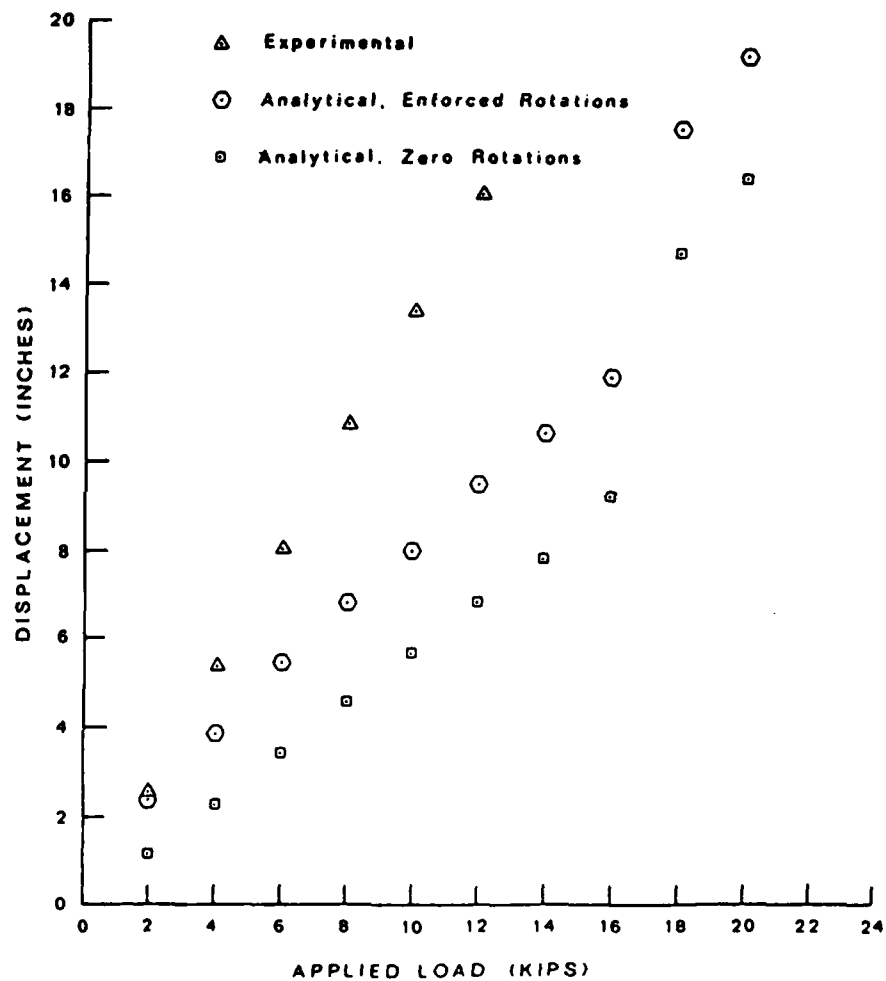
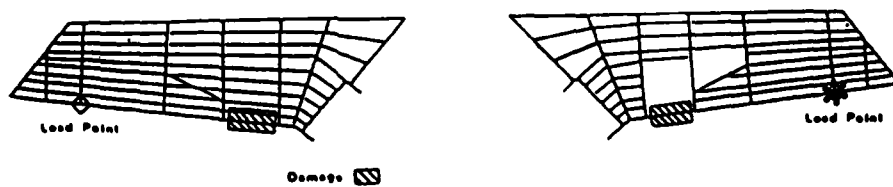
(c) Load Point, Model C (without torsional stiffness rods)

Figure 31. (Continued)



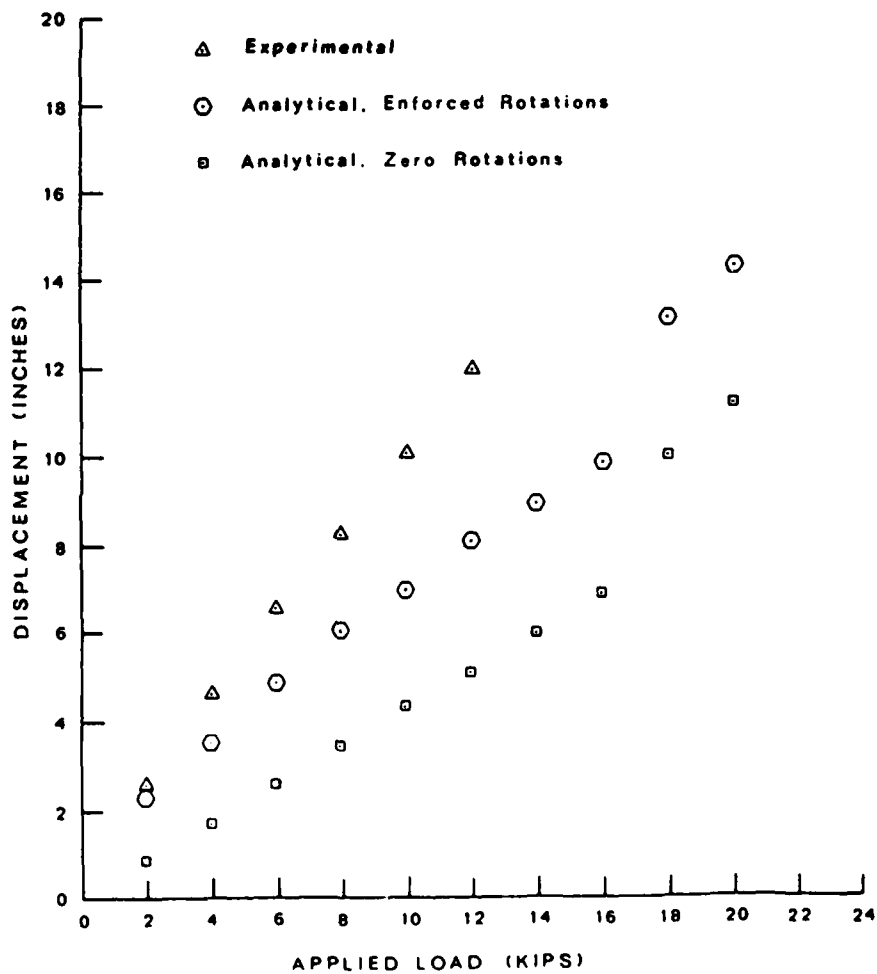
(d) Node 46, Model C (without torsional stiffness rods)

Figure 31. (Continued)



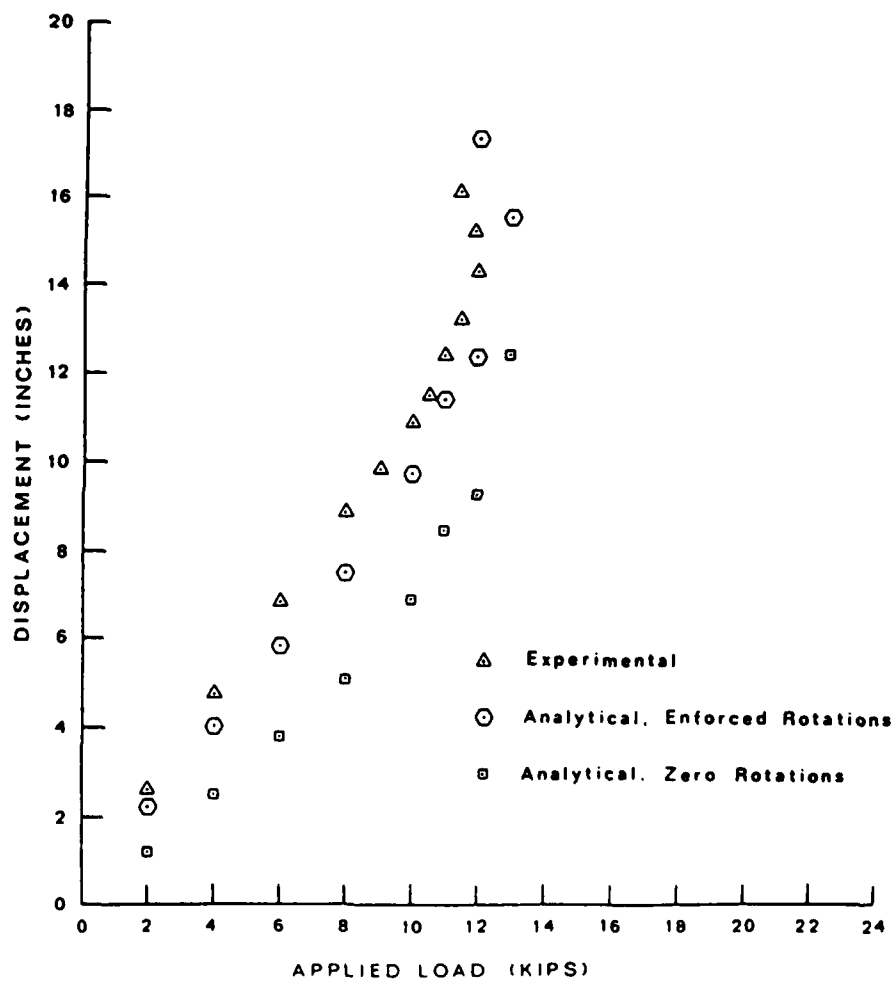
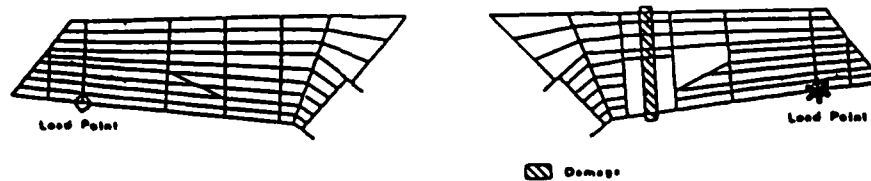
(e) Load Point, Model D (with torsional stiffness rods)

Figure 31. (Continued)



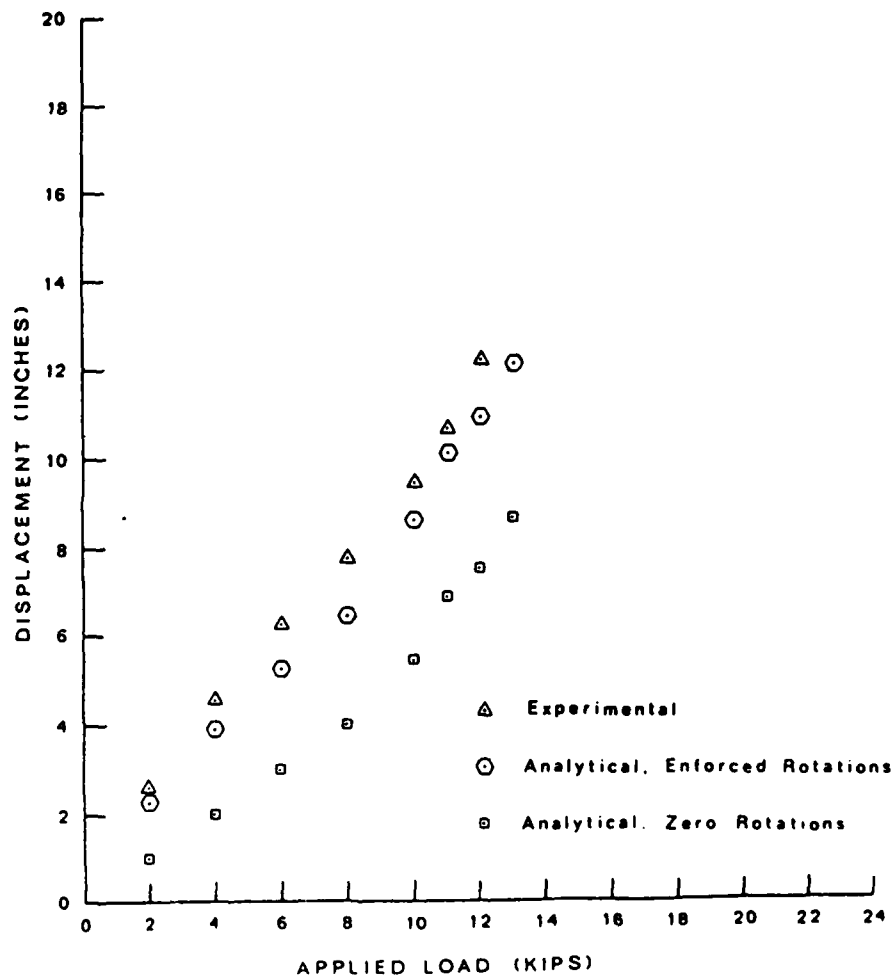
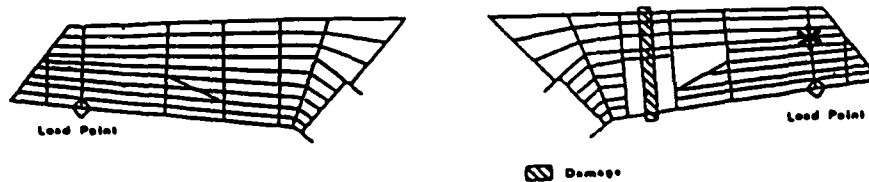
(f) Node 46, Model D (with torsional stiffness rods)

Figure 31. (Continued)



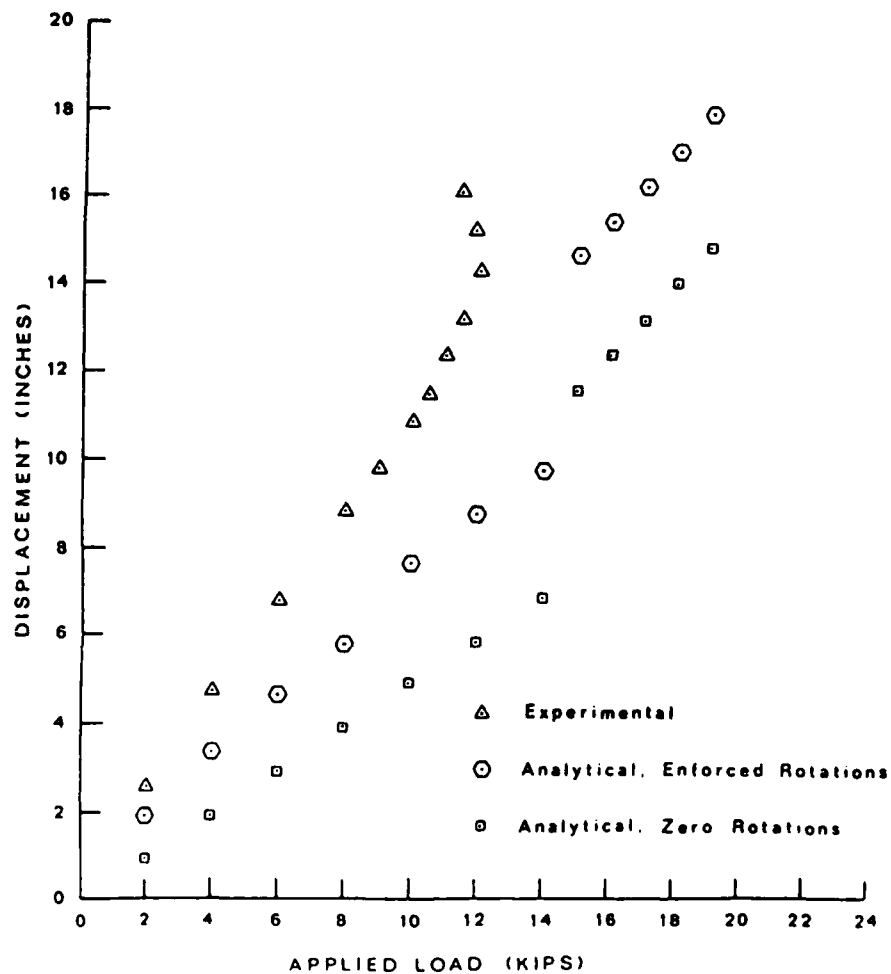
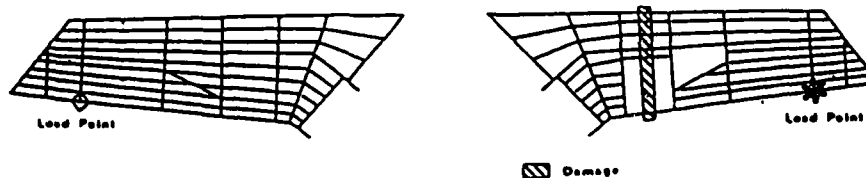
(a) Load Point, Model A (without torsional stiffness rods)

Figure 32. Single-Point Displacements for Test 3B



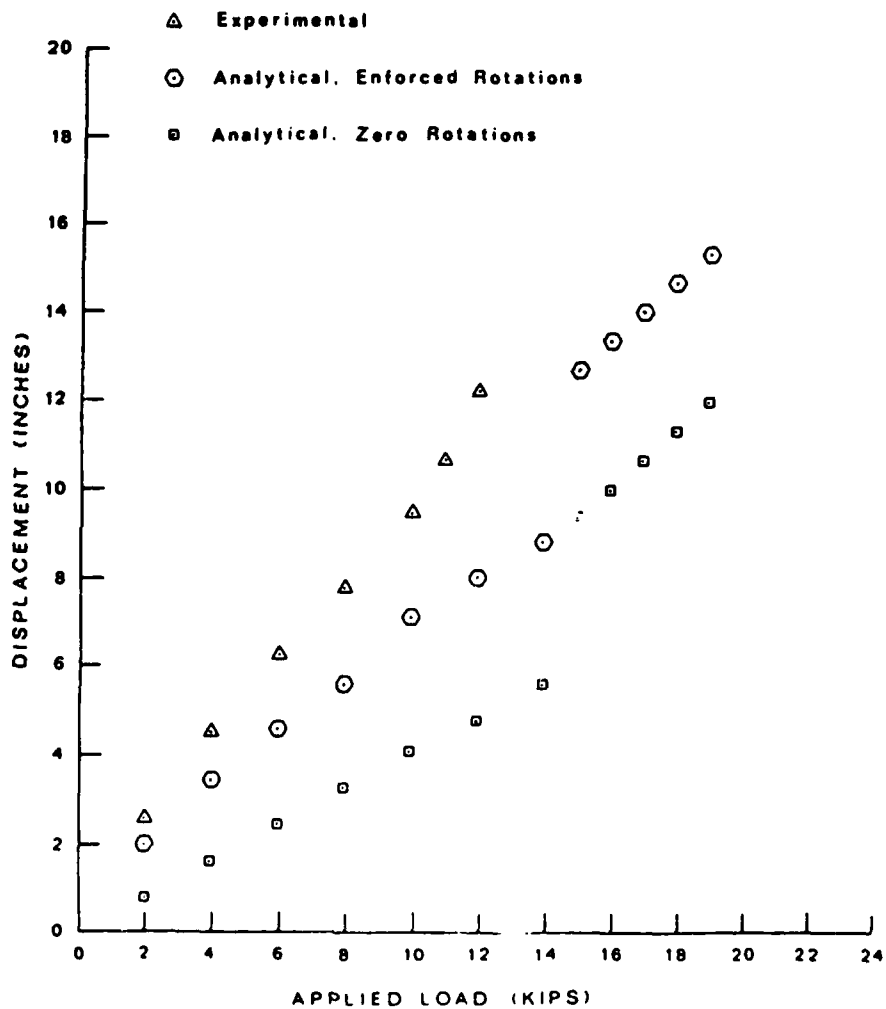
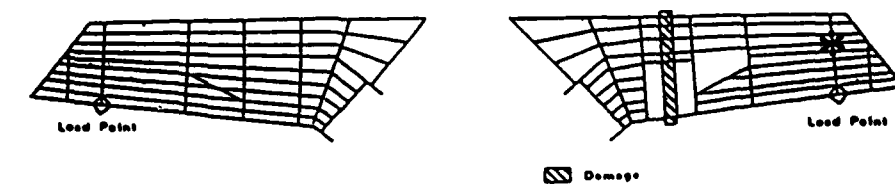
(b) Node 46. Model A (without torsional stiffness rods)

Figure 32. (Continued)



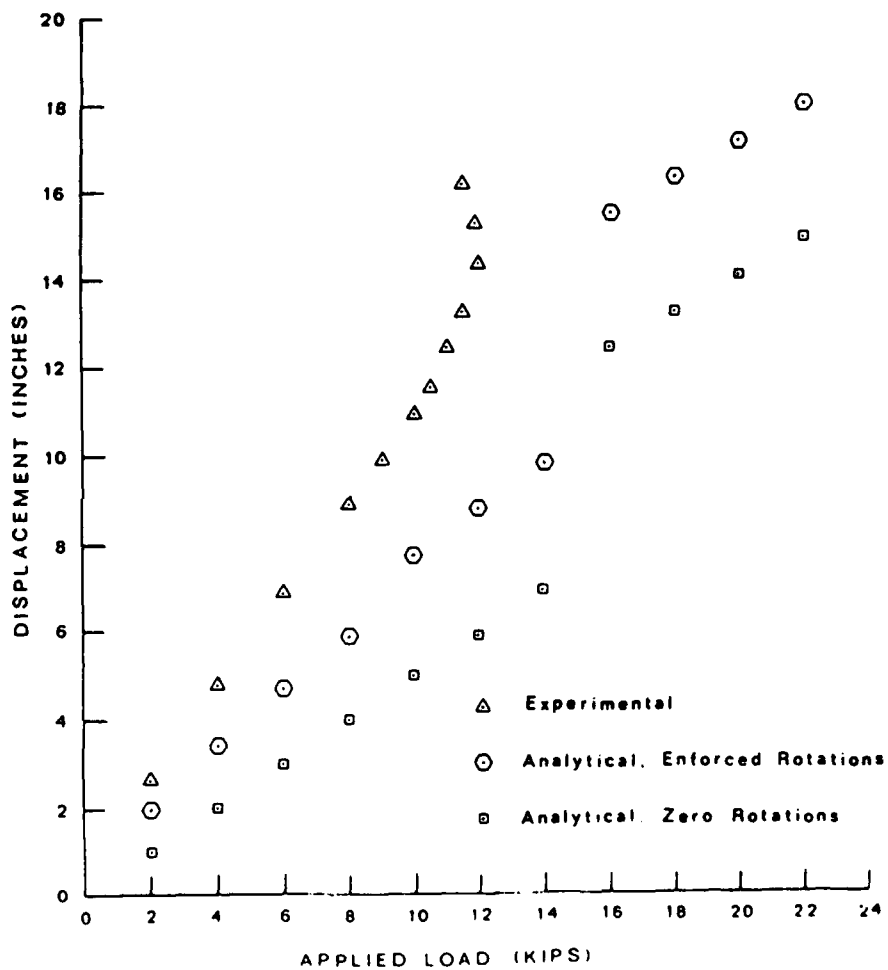
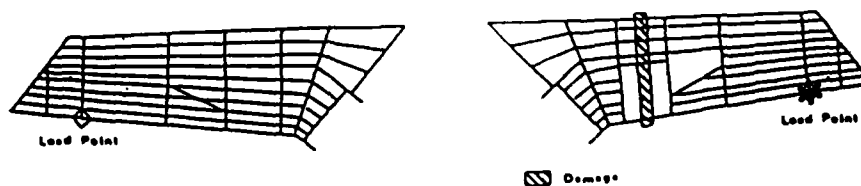
(c) Load Point, Model C (without torsional stiffness rods)

Figure 32. (Continued)



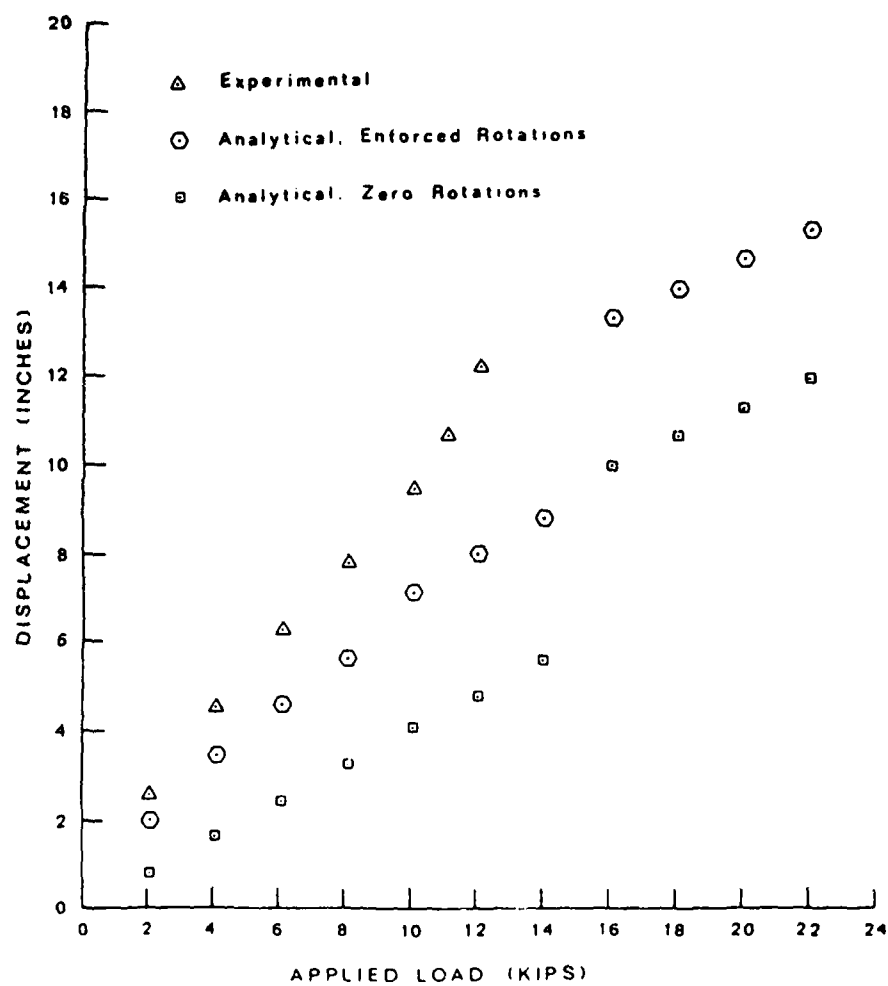
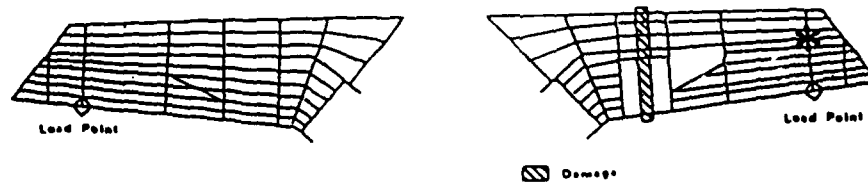
(d) Node 46, Model C (without torsional stiffness rods)

Figure 32. (Continued)



(e) Load Point, Model D (with torsional stiffness rods)

Figure 32. (Continued)



(f) Node 46. Model D (with torsional stiffness rods)

Figure 32. (Continued)

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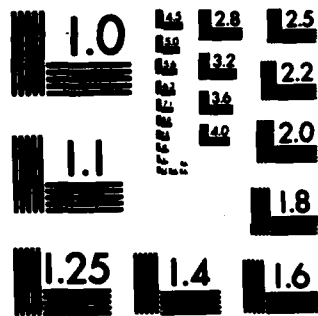
ANALYSIS OF PROGRESSIVE COLLAPSE OF COMPLEX STRUCTURES
(U) AIR FORCE INST OF TECH WRIGHT-PATTERSON AFB OH
G E RIGGS DEC 82 AFIT/CI/NR-82-63D

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APPENDIX J

SUMMARY OF ANALYTICAL RESULTS

TABLE V
SUMMARY OF RESULTS FOR TEST 1,
MODEL D, SIMPLE DAMAGE

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
1	10	68 118	---	---
2	12	56 112	---	---
3	14	136	---	---
4	16	54 80 106 124 135 255	529	529
5	14	---	---	---
6	16	---	---	---
7	18	82	---	---
8	20	111 117 132 134 258	---	---
9	22	64 116	117	117
10	20	---	137 573	573
11	18	---	119	119
12	16	---	135 571	135
13	14	---	531	531
14	12	---	---	---
15	14	---	---	---
16	16	105 257	---	---
17	18	85	105 121	105
18	16	123	533	533
19	14	---	---	---
20	16	---	123	123
21	14	---	---	---
22	16	---	---	---
23	18	---	579	579
24	16	---	---	---
25	18	---	---	---
26	20	---	---	---
27	22	52 62 89 107 125	75 248	248
28	20	93 114 143 256	107 125 213 257	125

TABLE V (Continued)

Iteration No.	Load	Buckled Elements	Overstressed Elements				Failed Elements
29	18	126	107	256	257	581	257
30	16	---	581				581
31	14	---	126				126
32	12	---	582				582
33	10	---			---		---
34	12	---			---		---
35	14	---			---		---

TABLE VI
SUMMARY OF RESULTS FOR TEST 1,
MODEL D, DETAILED DAMAGE

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
1	10	68 118	---	---
2	12	56 112 136	---	---
3	14	---	---	---
4	16	80 135	---	---
5	18	54 106 124 132	---	---
6	20	116 117 134 254 255 258	121	121
7	18	---	117 137	137
8	16	111	119	119
9	14	256 257	577	577
10	12	---	---	---
11	14	123	---	---
12	16	82 105 126	123 531 9529	123 9529
13	14	---	579	579
14	12	---	---	---
15	14	125	---	---
16	16	85 143	105 125	125
17	14	---	257	257
18	12	---	---	---
19	14	---	581	581
20	12	---	---	---
21	14	---	---	---
22	16	114	---	---
23	18	62 64 89	---	---
24	20	52 93	68 256	68 256
25	18	50 130	---	---
26	20	55	---	---
27	22	---	126	126
28	20	---	582	582
29	18	---	---	---

TABLE VI (Continued)

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
30	20	---	---	---
31	22	---	75	75
32	20	67 73	55 57 473	473
33	18	97	57 73	73
34	16	---	55 471	55
35	14	53	427	427

TABLE VII
SUMMARY OF RESULTS FOR TEST 2C,
MODEL A, SIMPLE DAMAGE

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
1	10	111 135	---	---
2	12	80	---	---
3	14	49 81 82 133 136 255	---	---
4	16	68 129 134	109	109
5	14	114 131	127	127
6	12	132	---	---
7	14	130	---	---
8	16	56	---	---
9	18	50 79 107 108	137	137
10	16	---	119 575	575
11	14	113 123 125 143 144	119 129 133 135 139 573 577 579	119 129 579
12	12	142	103 121 131 133 135 139 531 567 577 581 605 615	131 577 581
13	10	85 93 97 105 112 115 116 117 118 124 126 141 254 256 257	113 121 125 133 135 139 140 213 223 224 225 226 227 228 535 536 567 569 573 580 582 589 591 607 611 620 625	133 582
14	8	106	97 108 115 117 121 123 125 126 135 139 140 141 223 224 225 226 227 228 257 528 534 535 536 569 571 573 578 580 589 591 609 611 625 627 629 631 632	135 528
15	6	73	97 104 108 110 113 115 117 121 123 125 126 128 139 140 141 144 212 213 223 224 225 226 227 228 257 530 534 535 536 569 578 580 591 593 601	580

TABLE VII (Continued)

Iteration No.	Load	Buckled Elements	Overstressed Elements					Failed Elements		
15 (Cont.)			618	625	627	628	629			
			630	631	632					
16	4	---	97	104	111	115	117	111	552	578
			118	121	123	124	125			
			126	128	139	140	141			
			142	212	213	222	223			
			224	225	226	227	228			
			249	257	532	534	535			
			536	552	555	567	569			
			578	589	591	593	595			
			601	616	623	625	627			
			628	629	630	631	632			

TABLE VIII
SUMMARY OF RESULTS FOR TEST 2C,
MODEL D, DETAILED DAMAGE

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
1	10	56 68 80 112 114 118 126	---	---
2	12	106 132	---	---
3	14	62 82 111 135 256	---	---
4	16	49 50 108 124 134 255	---	---
5	18	116 117 133 258	68	68
6	16	---	---	---
7	18	---	66	66
8	16	79 81 113 129	460	460
9	14	---	---	---
10	16	---	462	462
11	14	---	---	---
12	16	---	---	---
13	18	61 105 107	70	70
14	16	54	---	---
15	18	52 115	---	---
16	20	---	56	56
17	18	63	430	430
18	16	---	---	---
19	18	---	58 432	58
20	16	64	432	432
21	14	---	---	---
22	16	---	---	---
23	18	---	62 255 434	62 255
24	16	254	64 78 81 82 187 434	81 434
25	14	---	64 78 79 82 105 254 436	78 79 254
26	12	55	64 77 82 105 436 477 479	64 479

TABLE VIII (Continued)

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
27	10	85	77 82 105 436 477 481	77 436 481
28	8	---	---	---
29	10	131	54 80 428 475 477	54 80 475 477
30	8	73 126	52 75 82 428 438	52 75 428 438
31	6	---	55 473	55
32	4	---	---	---
33	6	---	427 429	427 429
34	4	---	---	---

TABLE IX
SUMMARY OF RESULTS FOR TEST 3B,
MODEL A, SIMPLE DAMAGE

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
1	10	---	510	510
2	8	---	---	---
3	10	---	---	---
4	11	49 114 134 136	110	110
5	10	130 132	128	128
6	8	---	---	---
7	10	135	---	---
8	11	---	---	---
9	12	---	---	---
10	13	116 133	66	66
11	12	56	---	---
12	13	111	460 462	460 462
13	12	52 55 82	424 432	424 432
14	10	80 113	58	58
15	8	---	528	528
16	6	---	---	---
17	8	81 254	434	434
18	6	---	436	436
19	4	---	---	---
20	6	79 256	---	---
21	8	61 129 197 213	52 438 482	52 438
22	6	62	---	---
23	8	63 107 115 257	109 127 437 582	127 437 582
24	6	---	109	109
25	4	---	---	---
26	6	---	---	---
27	8	51	78 111	78 111
28	6	---	60	60
29	4	54	532	532

TABLE X
SUMMARY OF RESULTS FOR TEST 3B,
MODEL D, DETAILED DAMAGE

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
1	10	56 68 112 114 118 136	---	---
2	12	132 256	---	---
3	14	62 134	---	---
4	16	49 80 116 135	510	510
5	14	---	110	110
6	12	130	112 128	112
7	10	---	566	566
8	8	---	---	---
9	10	---	---	---
10	12	---	---	---
11	14	---	114	114
12	12	50 82	568	568
13	10	---	---	---
14	12	---	---	---
15	14	124	---	---
16	16	258	---	---
17	17	67 117	---	---
18	18	111 254	---	---
19	19	---	530	530
20	18	---	528 532 534 574	528 532 534
21	17	55 64 73 97 107 113 126 129 143 255 257	118 120 256 552 574	120 256
22	16	115 125	116 117 118 138 257 536 552 574 592	117 118 536
23	14	51 63 85 123 131 133 141	56 58 79 81 85 107 115 119 137 257 279 430 476 499 535 573 574	73 107 115 279 476 573 574

TABLE X (Continued)

Iteration No.	Load	Buckled Elements	Overstressed Elements	Failed Elements
24	12	105	55 56 57 58 69 71 75 78 113 119 197 429 430 467 471 473 535 571	71 113 119 535 571
25	10	---	55 58 75 78 79 80 105 111 121 197 429 430 467 471 473 478 569 575	105 111 467 473 569
26	8	---	58 75 77 78 80 109 197 429 430 471 533 567	58 109 471 533 567

VITA

Gregory Edward Riggs

Candidate for the Degree of

Doctor of Philosophy

Thesis: ANALYSIS OF PROGRESSIVE COLLAPSE OF COMPLEX STRUCTURES

Major Field: Civil Engineering

Biographical:

Personal Data: Born in Wichita, Kansas, April 13, 1950, the son of Mr. and Mrs. C. Edward Riggs.

Education: Graduated from Thomas A. Edison High School, Tulsa, Oklahoma, in May, 1968; received the Bachelor of Science in Civil Engineering degree from the United States Air Force Academy Colorado Springs, Colorado, in June, 1972; received the Master of Science degree in Civil Engineering from the University of Illinois, Urbana, Illinois, in February, 1973; completed the requirements for the Doctor of Philosophy degree at Oklahoma State University in December, 1982.

Professional Experience: Base civil engineering officer, U.S. Air Force, 1973-1975; staff civil engineering officer, U.S. Air Force, 1975-1977; Civil Engineering Instructor, U.S. Air Force Academy, 1977-1979; Civil Engineering Assistant Professor, U.S. Air Force Academy, 1979-1980.

Professional Organizations: Chi Epsilon, National Society of Professional Engineers, Society of American Military Engineers.

Professional Registration: Registered Professional Engineer in Colorado.